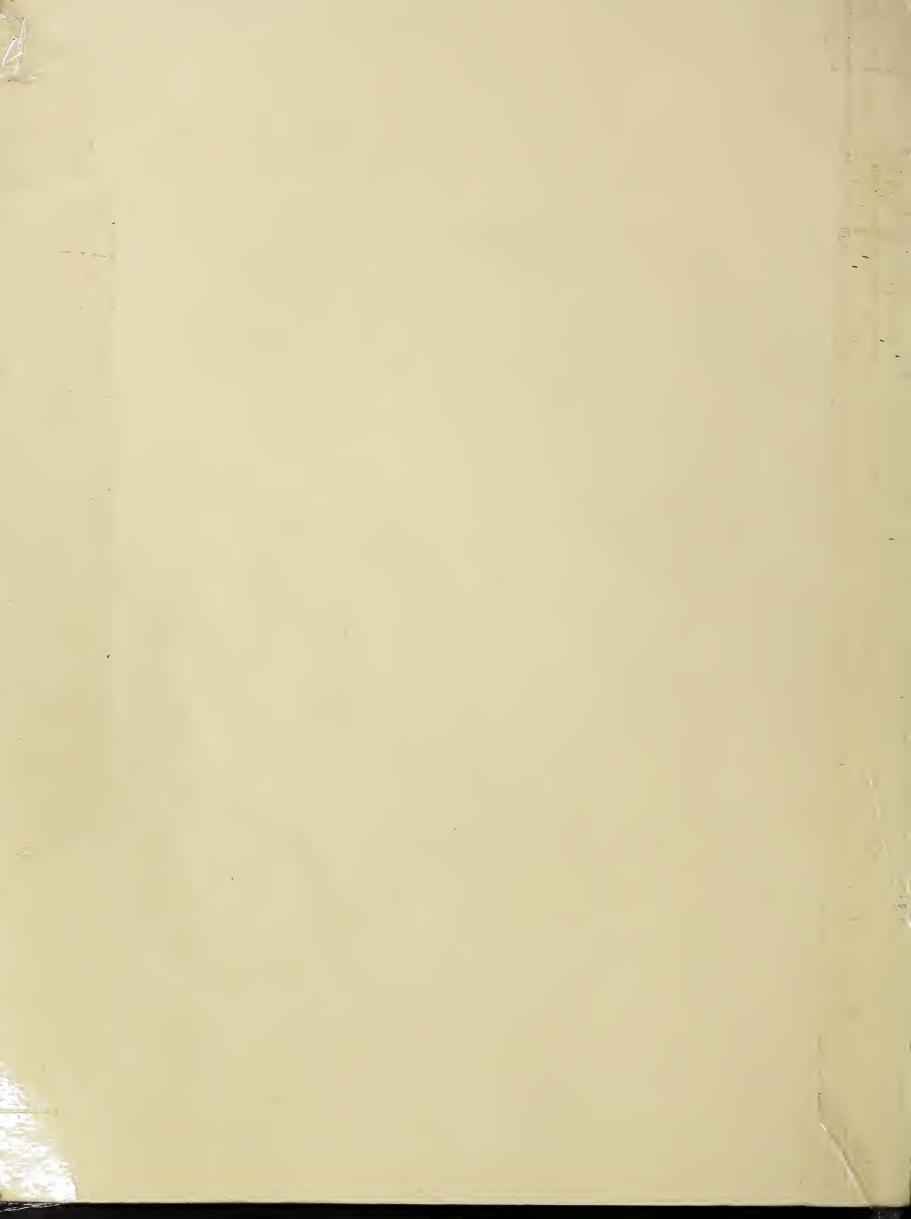
Historic, Archive Document

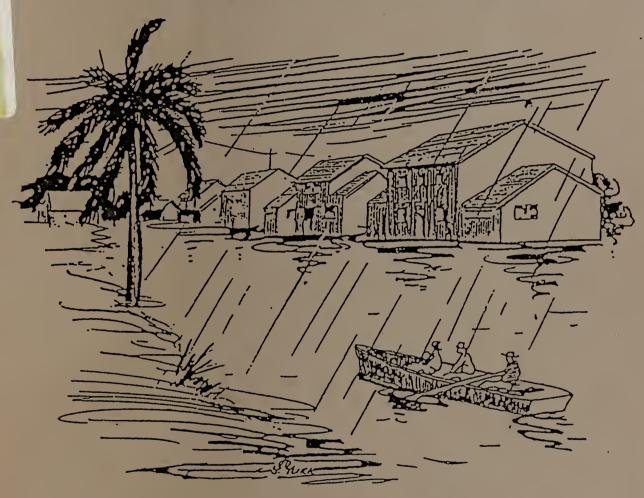
Do not assume content reflects current scientific knowledge, policies, or practices.



FLOOD PLAIN MANAGEMENT STUDY

LEE COUNTY, FLORIDA

POWELL, DAUGHTREY, POPASH, STROUD, MARSH POINT,
CHAPEL BRANCH, BAYSHORE, AND THOMPSON CUTOFF CREEKS
AND TRIBUTARIES TO YELLOW FEVER CREEK



Prepared By
U. S. DEPARTMENT OF AGRICULTURE
Soil Conservation Service
Gainesville, Florida

In Cooperation With
The Florida Department of Community Affairs
and
Lee County Soil and Water Conservation District
and
Lee County Commission
1984

1/0/85

Reserve

aTC424 .F6F6

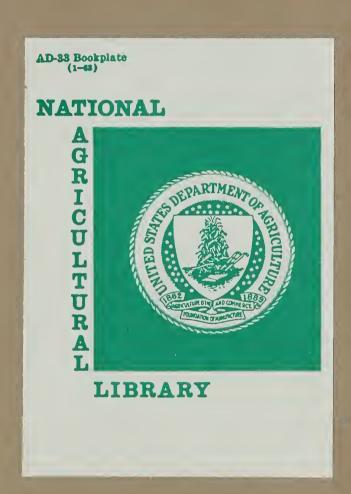


TABLE OF CONTENTS

	Page
LIST OF TABLES	iii
LIST OF FIGURES	iv
INTRODUCTION Requesting and Participating Entities Study Authorities Study Objectives	1 1 2 2
DESCRIPTION OF STUDY AREA	3
Location Stream System Geology Soils Climate Natural Values Land Use and Development Trends	3 5 5 7 8 9
FLOOD PROBLEMS	11
Flood History Flood Potential. Flood Hazard Photomaps Flood Profiles	11 12 13 14
FLOOD PLAIN MANAGEMENT ALTERNATIVES	15
Preventive Measures Corrective Measures Local Recommendations	16 17 20
GLOSSARY OF TERMS	21
BIBLIOGRAPHY	24
APPENDIX A (Flood Hazard Photomaps)	26
APPENDIX B (Flood Profiles)	33
APPENDIX C (Typical Valley Cross Sections)	56
APPENDIX D (Technical Appendix) Investigations and Analyses. Data Tables	60 61 62-80
Mas. Dept, of Agriculture	

NATIONAL AGRICULTURAL LIBRARY
'.14N 2 5 1988

CATALOGING = PREP



LIST OF TABLES

<u>Table</u>		Page
1	Temperature and Precipitation Data	8
2	Rainfall Frequencies	13
	Discharge, Elevation Frequency Data for:	
3	L-2 (Trib to Yellow Fever Creek)	62
4	L-1 (Trib to Yellow Fever Creek)	63
5	Powell Creek (with Suncoast Canal)	64
6	Powell Creek Trib (Old Railroad Grade)	65
7	Powell Creek Trib (US 41 Canal)	66
8	Marsh Point Creek	67
9	Marsh Point Creek Trib	68
10	Marsh Point East	69
11	Daughtrey Creek	70
12	Daughtrey Creek Trib (1)	71
13	Daughtrey Creek Trib (2)	72
14	Daughtrey Creek Trib (Daughtrey East)	73
15	Chapel Branch	74
16	Bayshore Creek	75
17	Bayshore Trib	76
18	Popash Creek	77
19	Stroud Creek	78
20	Thompson Cutoff	79
21	Thompson Cutoff Trib	80



LIST OF FIGURES

		Page
1. 2. 3. 4. 5. 6. 7. 8. 9. 10. 11. 12. 13. 14. 15.	Study Area Location Map with Hydrologic Boundaries General Soils Map of Study Area Photo of flooding Photo of construction. Photo of flooded house Photo of culvert w/sediment. Photo of improved road crossing. Photo of culvert system. Flood Hazard Photomap Index. Flood Hazard Photomap. Flood Profile Index Map.	4 6 7 10 12 18 19 19 27 28 29 30 28 31 32 34
F10 17. 18. 19. 20. 21. 22. 23. 24. 25. 26. 27. 28. 29. 30. 31. 32. 33. 34. 35. 36. 37.	L-2 (Trib to Yellow Fever Creek). L-1 (Trib to Yellow Fever Creek	35 36 37 38 39 40 42 43 44 45 47 48 49 50 51 52 53 55
Ty: 38. 39. 40. 41. 42.	pical Valley Cross Sections for: Popash Creek Cross Section POPO90 Powell Creek Cross Section POWO33 Powell Creek Cross Section POWO44 Marsh Point Creek Cross Section MARO03 Daughtrey Creek Cross Section DAUO60	57 57 58 58 59



INTRODUCTION

The information presented in this report was developed for use by local decisionmakers and the public in making flood plain management decisions. It is hoped that this information will assist with development decisions in such a way that future intensive rainfalls will result in minimal inconvenience to residents of the area.

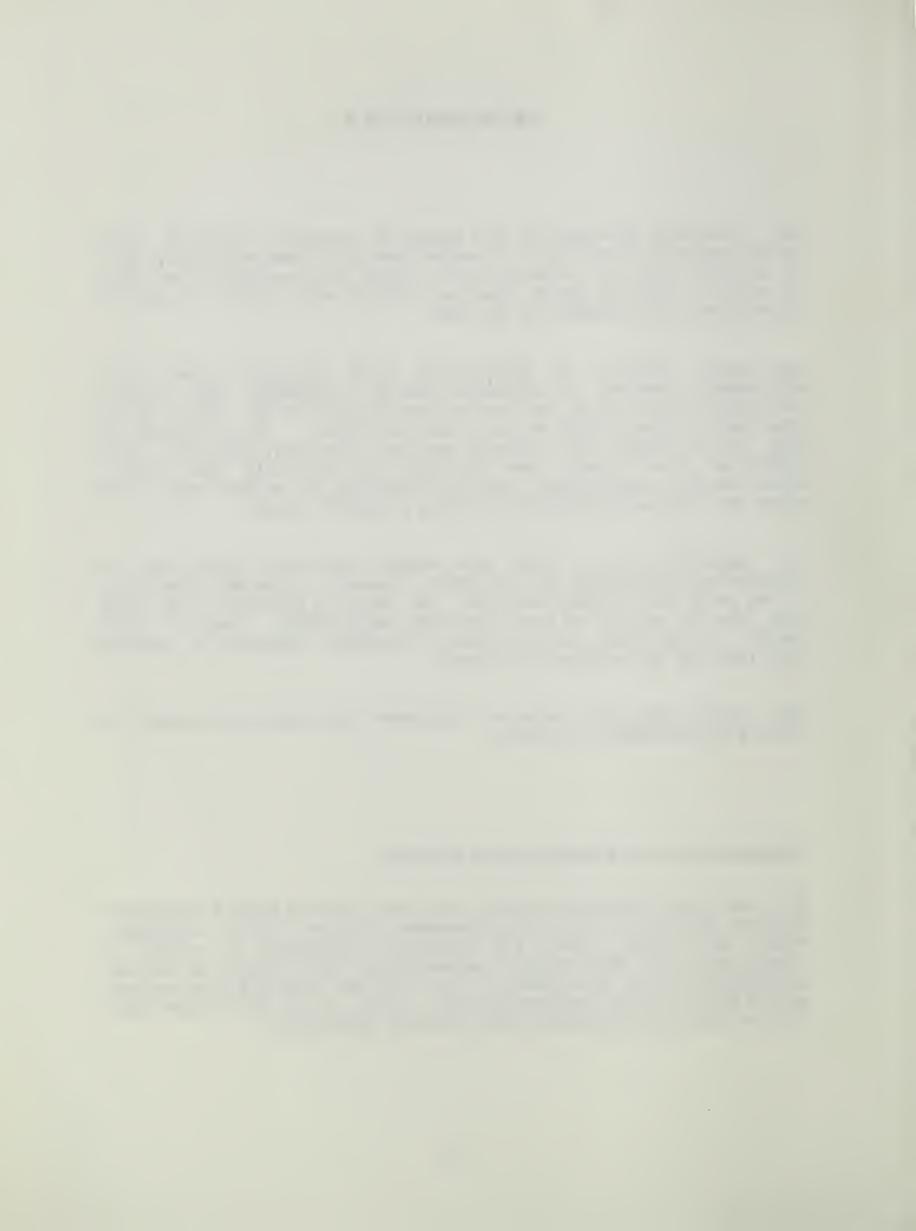
Lee County, Florida, is experiencing rapid population growth with accompanying demands for additional land to accommodate this growth. This study area, due to its location near the city of Ft. Myers, can be expected to undergo rapid growth in the near future. This is especially true in the southern and southwest portion of the study area where most properties have already been subdivided into relatively small units, where several subdivisions have been developed in recent years, and where very limited agricultural activity presently exists.

The majority of the study area remains basically rural and is comparatively unaffected by this growth; however, as growth intensifies in the south and southwest portions, the remaining portions of the study area can be expected to have increasing development. Since a large portion of the study area is subject to flooding, pressures to develop such lands can be expected to increase.

This report identifies the major flood-prone areas and will be useful in flood plain management decisions.

Requesting and Participating Entities

The Lee County Commission and the Lee County Soil and Water Conservation District requested a flood plain management study on Powell, Daughtrey, Popash and Stroud Creeks and associated tributaries to assist in identifying local flood problems and making decisions related to land use planning and future development. This study was conducted in accordance with a plan of study developed March 19, 1980, by the Soil Conservation Service (SCS) and requesting state and local authorities.



Lee County employees aided in gathering base data for the study. In addition, the County assumed payment for 50% of a contract for photogrammetric contour mapping.

Study Authorities

The SCS is authorized to provide technical assistance to federal, state, and local governing bodies in the development, revision, and implementation of their flood plain management programs by carrying out flood plain management studies (FPMS's) in accordance with Federal Level Recommendation 3 of "A Unified National Program for Flood Plain Management", and Section 6 of Public Law 83-566. This is in accordance with Recommendation 9(c) of House Document No. 465, 89th Congress, 2nd Session; Executive Order 11988 dated May 24, 1977; and USDA Secretary's Memoranda 1606 and 1607.

In Florida, these studies are authorized under the December 1978 Joint Coordination Agreement between the SCS and the Florida Department of Community Affairs. The Department Secretary is under the direction of the Governor of Florida and is responsible for receiving requests, setting priorities, and coordinating flood plain management studies conducted by the SCS and other state and federal agencies.

Study Objectives

An immediate need exists in the study area to accurately define the existing flood hazard areas so that local governments may plan and carry out an effective flood plain management program. The objective of this flood plain management study is to furnish technical information to the Lee County Board of County Commissioners and the Lee County Soil and Water Conservation District in the form of maps, graphs, and tables depicting various flood discharge and elevation frequency data. This flood plain information is needed as a basis for local flood plain management and land use programs so as to reduce flood losses and enhance the environment of natural flood plain areas.



DESCRIPTION OF STUDY AREA

The study area is a part of the Daughtrey-Trout Creek Watershed characterized by a complex hydrological system. The area is threatened with urban development because of its numerous desirable attributes.

Location

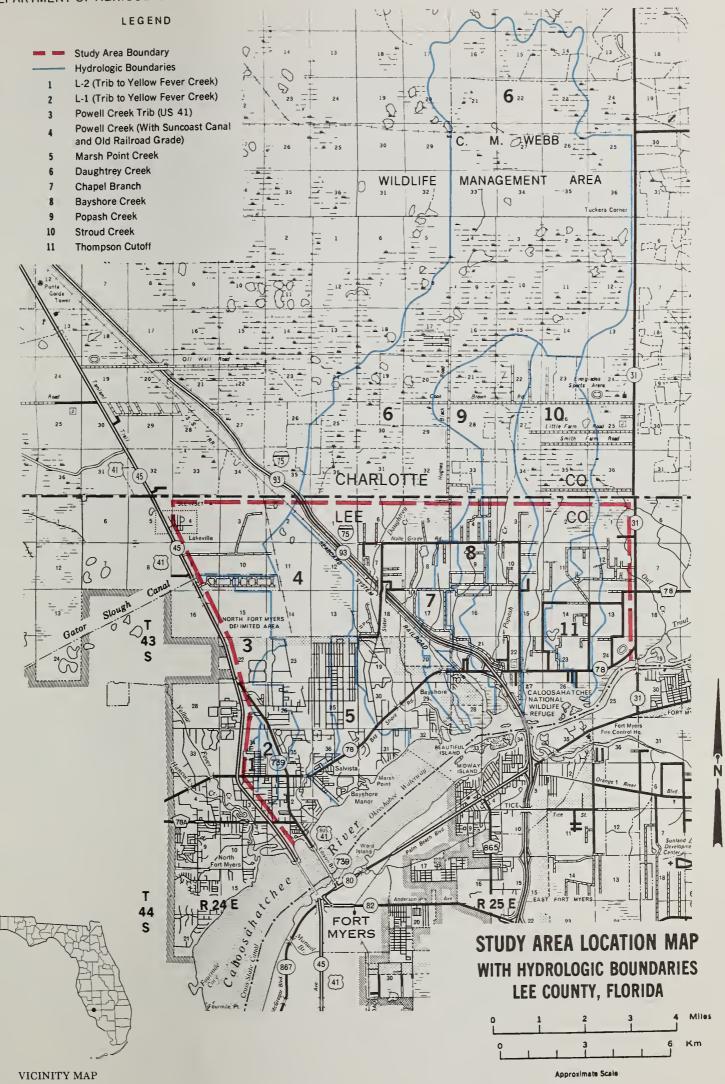
Located on the north side of the Caloosahatchee River, the study area consists of that part of the Daughtrey-Trout Creek Watershed east of U. S. Highway 41 and west of State Road 31 and north to the Lee-Charlotte County line. The area is approximately 50 square miles. Figure 1 shows the study area as well as the stream system.

Stream System

The study area is located within the United States Geological Survey's (USGS's) hydrologic unit number 03090205. The average stream temperature is between 72° and 76° F. The largest fresh water uses are irrigation and municipal water supply. Most of this water is obtained from the upper Floridan Aquifer which has a water hardness in excess of 181 chemical PPM (parts per million).

The study area has 10 tributaries flowing into the north side of the Caloosahatchee River with five having one or more first order tributaries for a total of nine. Approximately 49 miles of tributaries have been analyzed. The hydrologic boundaries of the tributaries and contributing drainage areas are largely indeterminate and subject to change because of the flat topography and swampy conditions. Approximately 67 square miles of headwater area is in Charlotte County to the north of the study area (see Figure 1), making the total drainage area 117 square miles. Part of this headwater area is within the C. M. Webb Wildlife Management Area.







The Caloosahatchee River is tidal from its mouth in San Carlos Bay to approximately 25 miles upstream, including the study area. At North Fort Myers the tidal range is approximately one foot.

During the rainy season, June through September, the water table usually ranges from 4 feet below ground surface up to or even above the ground surface. During the drier winter months it is from 2 to 6 feet below ground surface. Drainage canals may lower and surface impoundments may raise water levels in underground aquifers.

Geology

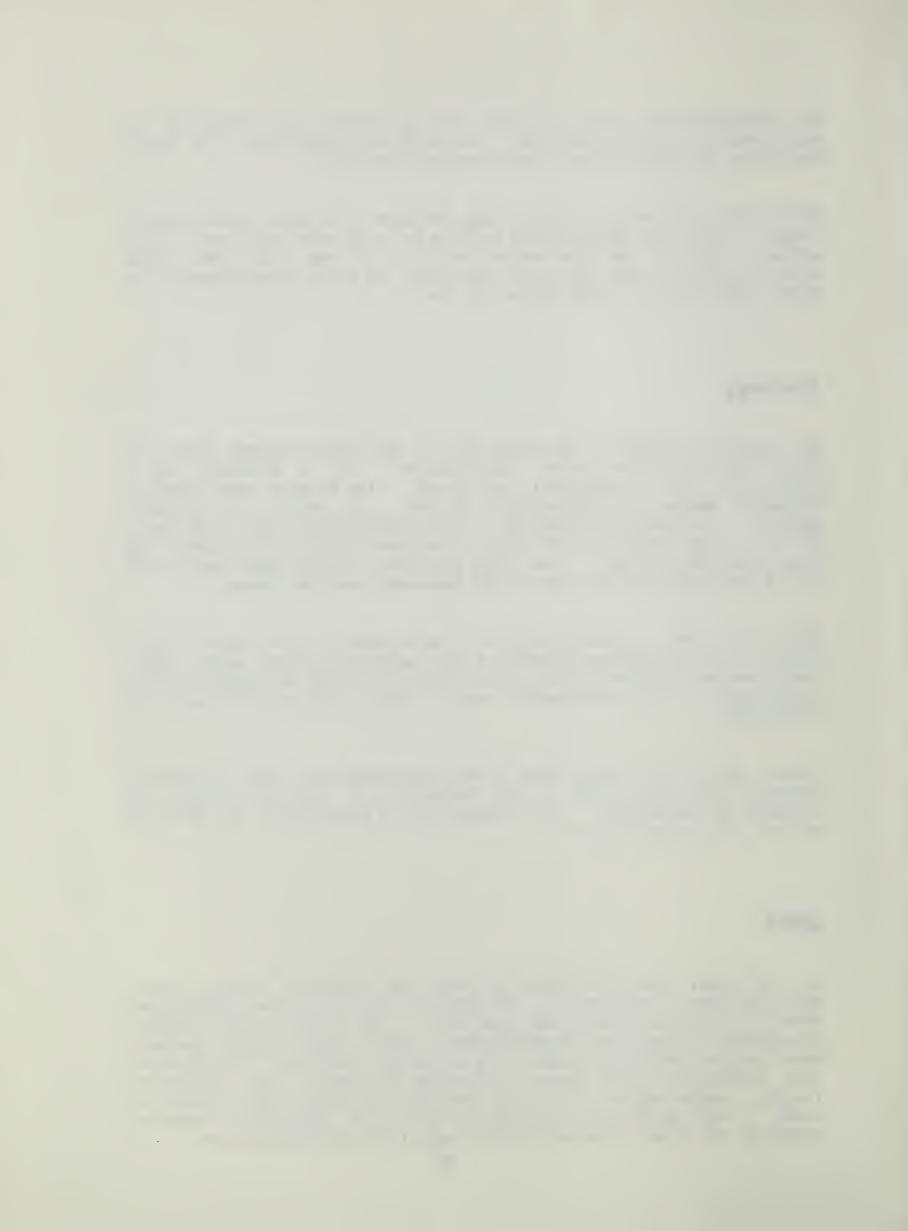
The surface geology of the study area is all recent unnamed sand and shell containing the water table aquifer. It is underlain by a Pleistocene aged unnamed marl shelly sand. The Miocene aged Tamiami Formation begins at approximately 40 feet. It contains a sandstone aquifer. Ine Hawthorn Formation is from approximately 150 to 550 feet. It is a light gray to white shelly limestone containing the upper and lower Hawthorn aquifers. The Tampa Limestone is from approximately 550 to 775 feet where the Oligocene aged Suwannee Limestone begins.

There has been a gradual decline of water level in the water table aquifer due to increased pumping of the sandstone aquifer since 1969. There are two USGS observation wells within the study area which monitor fluctuations in the ground water levels. Annual reports are issued with these data.

Ground elevations range from 38 feet above mean sea level (National Geodetic Vertical Datum) in the headwaters in Charlotte County to tidewater in Lee County. The average drop in elevation is 2.7 feet per mile or .05 percent slope.

Soils

In the study area (see Figure 2) there are primarily nearly level, poorly drained, sandy soils. The General Soils Map of Lee County shows three soil associations in the study area. The largest is the Pineda-Boca-Wabasso Association characterized by nearly level, poorly drained, deep loamy and moderately deep loamy soils over limestone. The soils of this association have severe limitations for septic tank absorption fields, sewage lagoons, sanitary landfills, dwellings, and streets and roads. They have very low potential for citrus production, moderate potential for truck crops and high potential for improved pasture.



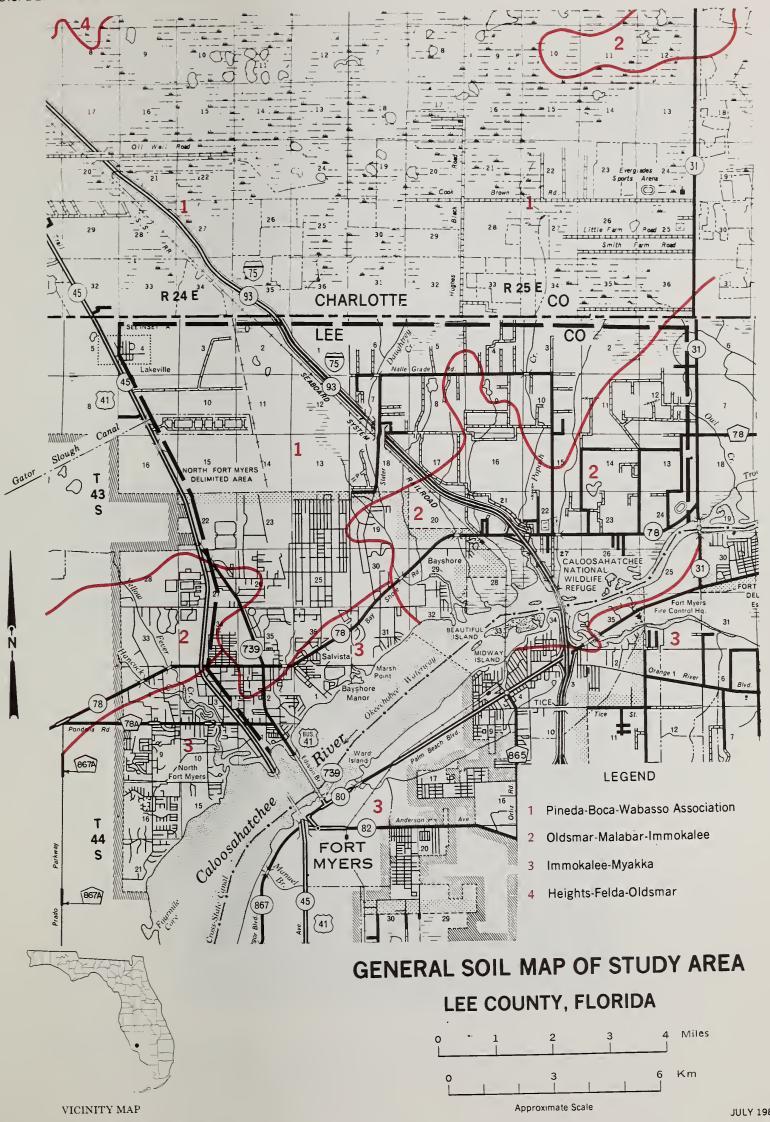


Figure 2



The second largest soil association found in the study area is the Oldsmar-Malabar-Immokalee Association. It is characterized by having nearly level, poorly drained soils that have a dark, organic stained subsoil. Some soils are sandy throughout and some subsoils are loamy below a depth of 40 inches. A small area is the Immokalee-Myakka Association which is characterized by nearly level, poorly drained soils that have a dark, organic stained subsoil underlain by sandy material.

The General Soils Map of Charlotte County shows the headwater area as being mainly of the Pineda-Boca-Wabasso Association with some small areas of the Oldsmar-Malabar-Immokalee Association and another nearly level poorly drained soil.

The SCS has recently completed a soil survey of Lee County. Copies of this report are available at the local SCS office.

Climate

The study area has a subtropical climate with an annual rainfall of 54 inches and an average temperature of 74°F (see Table 1). The wet season is from June through September and coincides with the hurricane season. During this 4-month period two-thirds of the annual rain occurs. All of the annual precipitation, except for 10 inches, will be lost either through evapotranspiration or deep seepage. Ten inches per year are available for runoff, mostly in the form of overland flow. The average growing season exceeds 320 days with from 325 to 350 frost-free days per year.



Figure 3. Flooding as a result of a monthly total of 9.45 inches of rain in September 1964. The largest 24-hour storm was only 2.78 inches.



Precipit	tation	Normals
----------	--------	---------

JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC ANNUAL 1.64 2.03 3.06 2.03 3.99 8.89 8.90 7.72 8.71 4.37 1.31 1.30 53.95

Mean Temperature

JAN FEB MAR APR MAY JUN JUL AUG SEP OCT NOV DEC ANNUAL 63.5 64.7 68.5 75.3 77.7 81.1 82.5 82.8 81.6 76.4 69.4 64.8 73.9

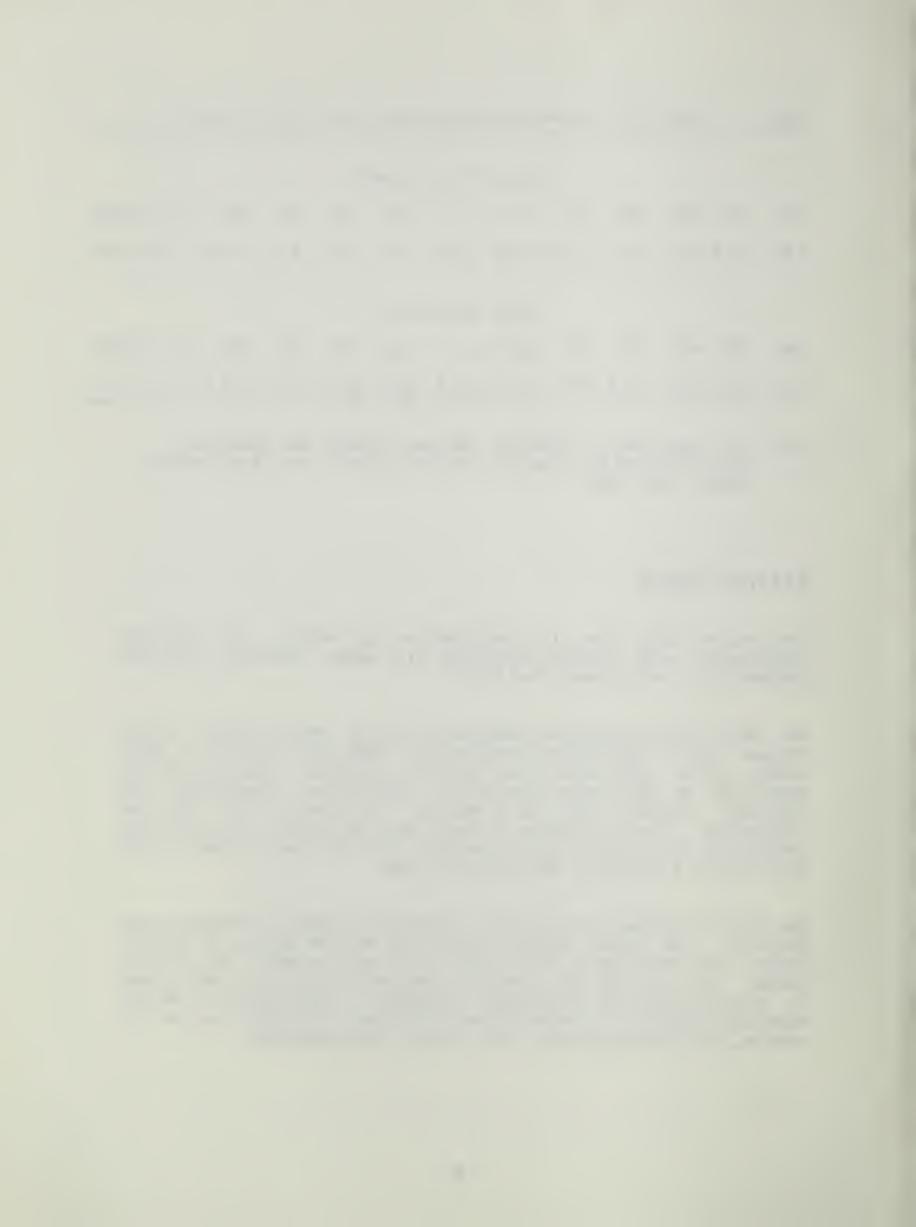
From: U.S. Department of Commerce, National Oceanic and Atmospheric Administration Environmental Data Service for Fort Myers Weather Station 1941-1970.

Natural Values

The entire study area is characterized by broad, low flatwoods interspersed with sloughs and marshes with waters generally draining southward to the Caloosahatchee River.

The south Florida flatwoods community occurs on nearly level, poorly drained soils. During the rainy season, these soils have high water tables, often with water at or above the surface. Typical natural vegetation on these areas consists of slash pine, sawpalmetto, and perennial grasses such as wiregrass, bluestems, and lopsided indiangrass. The flatwoods were logged over in the early part of this century and the grazing-burning practices since then have helped to keep this area in a relatively open savannah type.

The broad drainageways through the flatwoods are known as sloughs or wet prairies. The slough community appears as an open expanse of grasses, sedges and rushes where the soil is saturated throughout the growing season. Most sloughs are relatively long and narrow and slightly lower in elevation than the surrounding flatwoods. Characteristic natural vegetation consists of grasses (blue maidencane, bluejoint panicum, low panicum, and sand cordgrass), beak-rushes, and sloughgrass.



Depressional areas within the sloughs are occupied by the freshwater marsh vegetative communities. These are very poorly drained areas where the soil is saturated or covered with water for months during the growing season. Characteristic plants occurring in these marshes include maidencane, pickerelweed, arrowheads, sawgrass, fire flag, and cattail.

Where there are defined streams, the natural vegetation in the areas where these join the Caloosahatchee is characteristically in mangrove swamps.

The interspersed flatwoods, sloughs, and marshes support a large variety of wildlife. Mammals include raccoon, otter, opossum, skunk, marsh rabbit, armadillo, deer, bobcat, and feral hogs. Birds include bobwhite quail; several owls, hawks, and woodpeckers; numerous songbirds; and a large variety of wetland birds such as herons, egrets, ibis, bitterns, sandhill cranes, gallinules, and Florida ducks. There are a variety of frogs, turtles, and snakes, with alligators in the larger marshes and ponds.

Endangered or threatened species that occur or whose range indicates they might occur in the area include the alligator, indigo snake, wood stork, perequine falcon, ivory-billed woodpecker, red-cockaded woodpecker, bald eagle, southeastern kestrel, Florida sandhill crane, and Florida panther.

The fisheries resource includes species such as largemouth bass, several species of sunfish, pickerel, catfish, small minnows, bowfin, and gar. Additional species from saltwater areas are found in the mangrove swamps, which are renowned as nursery areas. Few large fish are produced, but the population explosion of small individuals that occurs each rainy season when the habitat expands, serves as the base of the food chain for many of the other animals occurring in the area.

Land Use and Development Trends

Due to its mild climate and other natural values and proximity to the Gulf beaches, the study area has experienced a rapid population increase over the past 12 years. The 1970 census showed Lee County having a population of 105,200. The 1982 census showed 227,300, a 116 percent increase! Lee County is the 12th most populated county in Florida and accounts for 2.19 percent of the State population. It is ranked 13th in population density in the State with 283 persons per square mile. Population projections are 257,700 for 1985, 311,000 for 1990, 400,600 for 2000, and 508,500 for 2020.

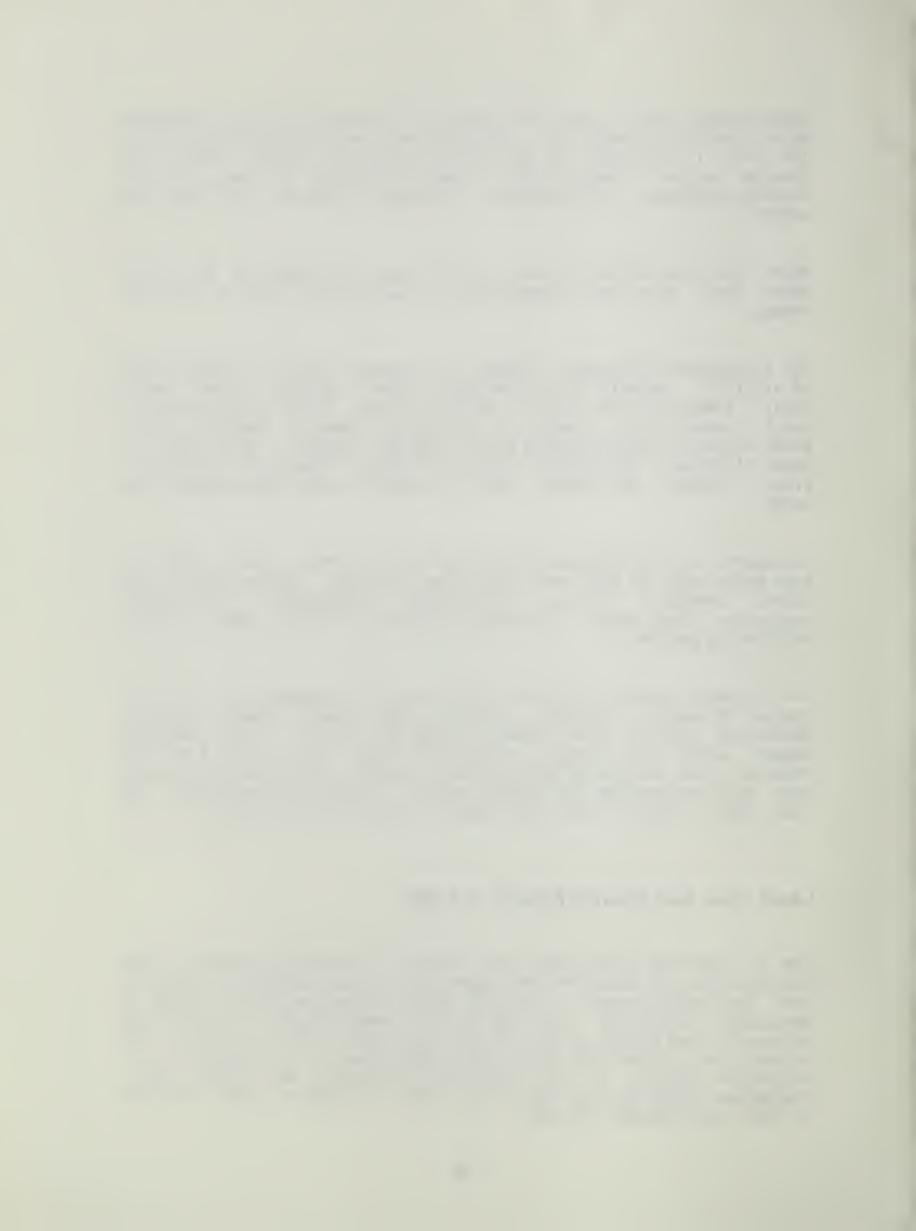




Figure 4. Construction in flood prone area

In 1980, Lee County had 13,257 non-permanent residents. Out of 111,013 total housing units, 2,415 were seasonally vacant. Twenty-six percent of all units are occupied by renters with 74 percent being owner occupied. The average household size is 2.46 persons. In fiscal year 1981 there were 26,701 mobile home tags sold in Lee County and 4,303 parcels of real property. In 1979, 33.8 percent of all housing units were mobile homes.

Most of the forest land in the study area is in slash pine growing in scattered patches intermingled with range. The stands are poorly stocked and growing on areas of poor site quality. No timber is being harvested in the study area and the nearest pulpwood yard is 50 miles away.

There are 15 million pounds of commercial shellfish harvested in Lee County each year and 10 million pounds of other fish.

Other agricultural land uses in Lee County include watermelon, cucumber, and pepper production which employs approximately 645 persons annually. Ornamental horticulture production employs 1,263 persons. Related agricultural services employs 617 persons. A smaller agricultural land use is pasture for beef and dairy production. The total annual net farm income for Lee County was \$4,750,000 in 1980.



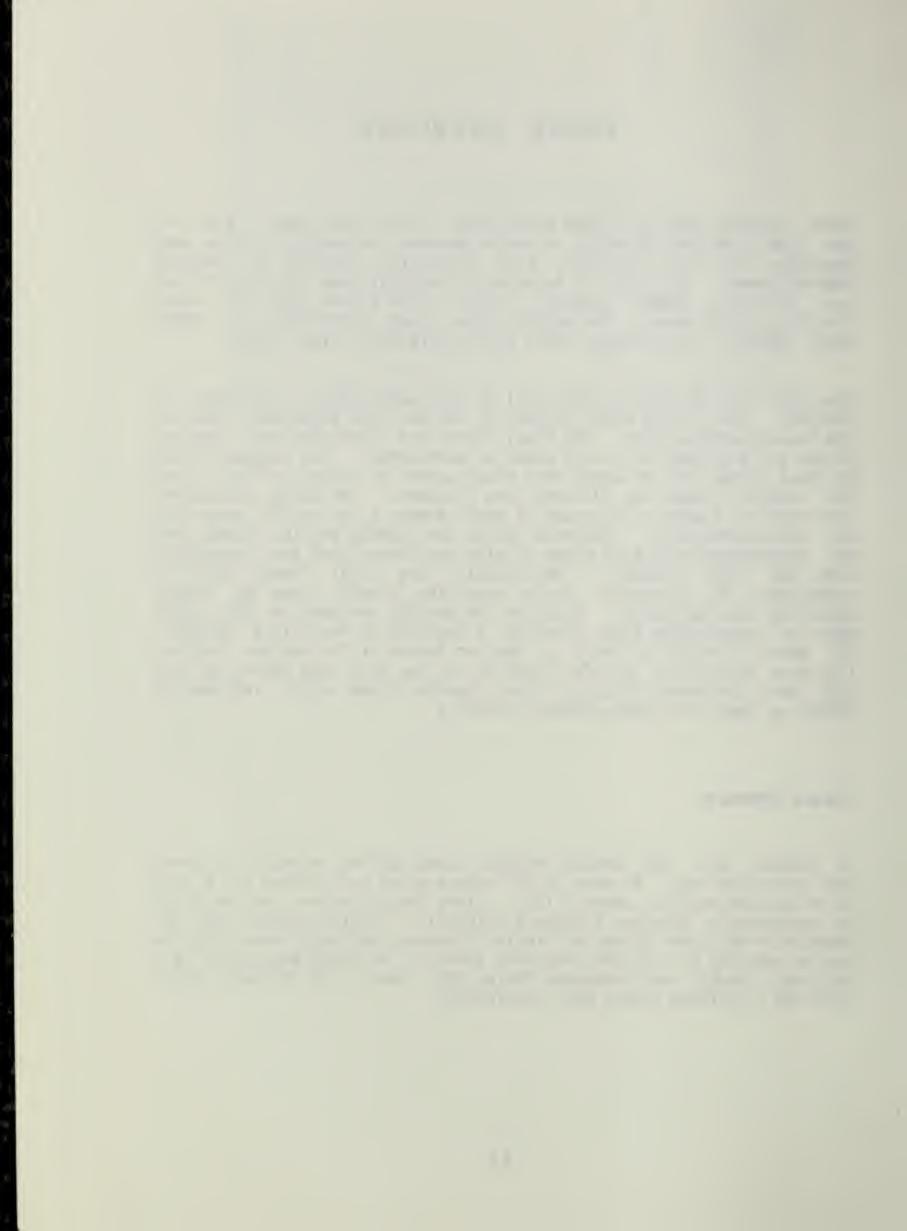
FLOOD PROBLEMS

There are two types of floods which occur in the study area. Most of the floods are from rainfall occurring between the months of June and September as short duration, high intensity afternoon or evening thundershowers. Rainfall for December through May generally occurs from less frequent, longer duration frontal type storms and may cause flooding in the area. The rainfall type flood is strictly of a fresh water nature. This report deals with the rainfall type flood.

The other type of flood is the tidal or salt water type. It is due to abnormal rises in the water surface of the Gulf and subsequent rises in the Caloosahatchee River. The tidal floods are associated with tropical storms and accompanied by high winds or hurricanes. The damages caused by tidal floods are far worse than those caused by rainfall floods, but the rainfall floods are 10 times more frequent. The damage associated with rainfall floods is a result of water damage alone and is generally not life threatening. The water moves very slowly and the floods are not accompanied with high winds as with hurricanes that are associated with the tidal floods. Occasionally the tidal floods will be accompanied by torrential rains resulting in both types of floods occurring simultaneously. A system for warning residents of the study area of approaching tidal flooding is operated at Page Field Airport, Ft. Myers, Florida, by the U. S. Weather Bureau at the weather station. (For more information on tidal flooding see the July 1980 report by the USGS Water Resources Division titled "Special Flood Hazard Information Report on South Lee County Coastal Areas".)

Flood History

In October 1924, the largest recorded flooding from rainfall occurred over the study area. An area of 350 square miles was flooded for 5 days to an average depth of about 1 foot. Since this flood was the result of an approximate 100-year frequency rainfall, similar results can be expected from future storms of similar frequency and more severe results can be expected from higher frequency storms. Although most roads and railroad tracks were inundated during this flood, the Seaboard Coast Line (SCL) railroad tracks were unaffected.



Other major floods from rainfall occurred in June 1901, June 1912, September 1935, and September 1962. The 1901 and 1912 floods were almost as large as in 1924. Since 1962, there were seven storms that exceeded the 2-year frequency of 5.0 inches but were less than the 5-year frequency of 6.2 inches.

The worst tidal flood occurred in October 1921 during the most severe hurricane recorded for the study area. The tide rose to 9 feet at Ft. Myers and covered the coastal islands. Other major tidal floods occurred in 1910, 1926, 1944, 1946, and 1960. Both the 1921 and the 1926 floods and associated hurricanes caused over \$1 million in damage. With the increase in urbanization and associated increases in property values, similar flood events would have substantially greater damages today.

Flood Potential

Seasonal flooding is common in parts of the study area. During periods of intense or prolonged rainfall, particularly during the summer rainy season, the water table rises above ground surface and begins to flow overland, slowly southward toward the Caloosahatchee River. The soil becomes saturated and the natural sloughs and ponds fill. These slightly flooded conditions can last for 30 days or more. Some problems can occur as a result of this type of yearly flooding.



Figure 5. House was built on raised pad. Damage inside was slight from this one-year frequency storm.



Even when houses are built on earth pads high enough to avoid letting water in, often times driveways and other parking areas, storage buildings, yards, patios and septic systems are not built on a sufficiently high area to avoid flooding. To some families, it may be a major inconvenience not to have the use of their car or yard for several days or even weeks, but a flooded and likely malfunctioning septic system can cause a health threat to the entire community. Problems resulting from this type of flooding are largely the result of uncontrolled and uncoordinated development.

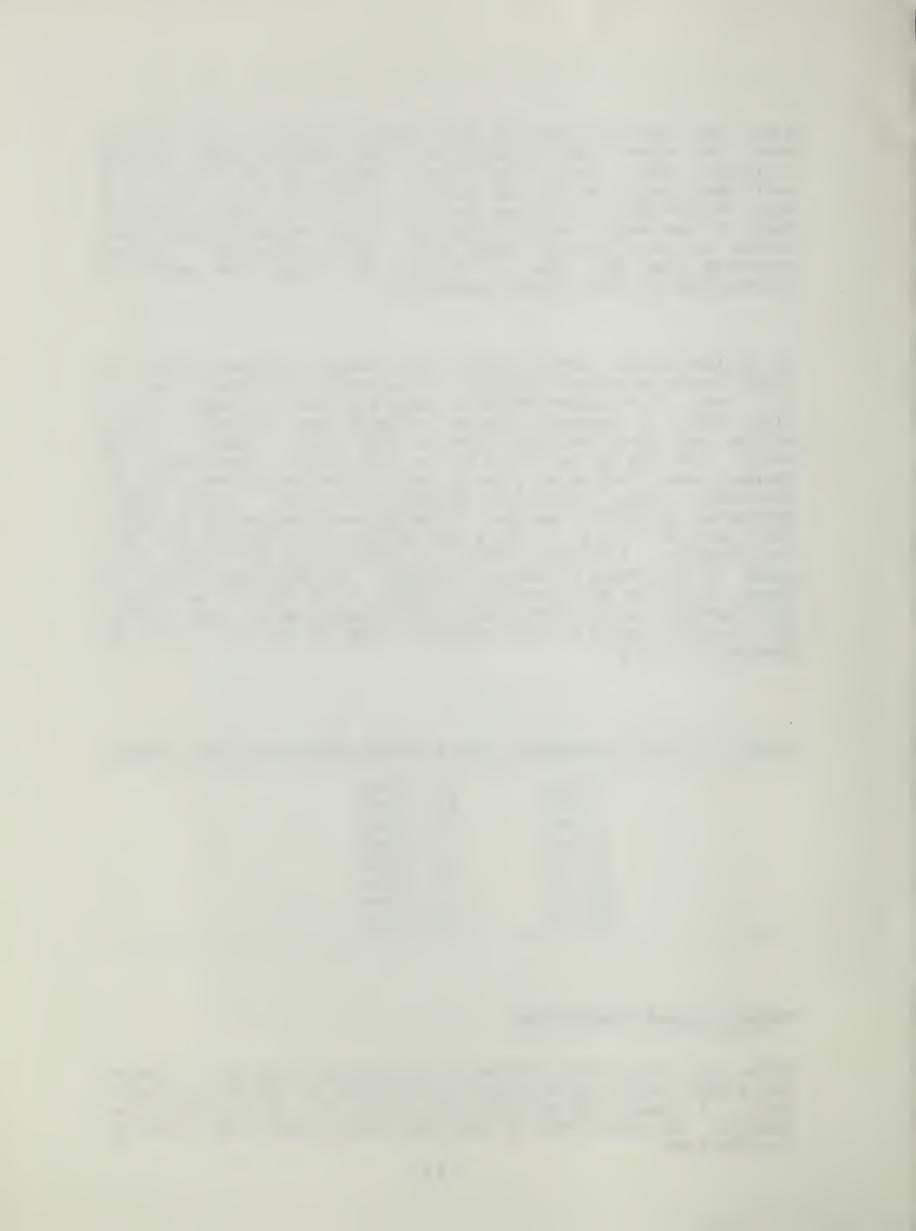
In addition to this yearly flooding larger storms occasionally occur. A flood having an average frequency of occurrence on the order of once in 100 years (a one percent chance of being equalled or exceeded in any given year) is generally used for criteria when designing highway However, floods bridges and other structures within a flood plain. larger than the 100-year flood can and will occur. Even though the maximum known flood on any given stream may have been extremely severe, eventually a larger flood can and probably will occur. In this study, floodwater elevations and peak discharges were generated for the 500-, 100-, 50-, 25-, 10-, 5-, 2- and 1-year rainfall return period for a 24hour duration. A 100-year rainfall can result in an even larger flood event if the ground is already saturated. The magnitudes of each of these floods were determined by an analysis of the rainfall and runoff characteristics of the contributing drainage areas and by flood routing. The rainfall depths of flood producing storms for the study area are presented in Table 2.

Table 2. Rainfall Frequencies (For a 24 hour storm)-Lee County, Florida

25-year 8.7 inches 50-year 10.0 inches 100-year 11.0 inches 500-year 13.5 inches	l-year 2-year 5-year 10-year 25-year	5.0 6.2 7.3	inches inches inches inches	
	50-year			

Flood Hazard Photomaps

There are 6 flood hazard photomaps in this report (Appendix A) showing the areas flooded by the base rainfall flood or 100-year frequency flood. A flood hazard photomap index (Figure 9) is also located in the appendix. The shaded areas on these maps are projected to be flooded by the base flood.

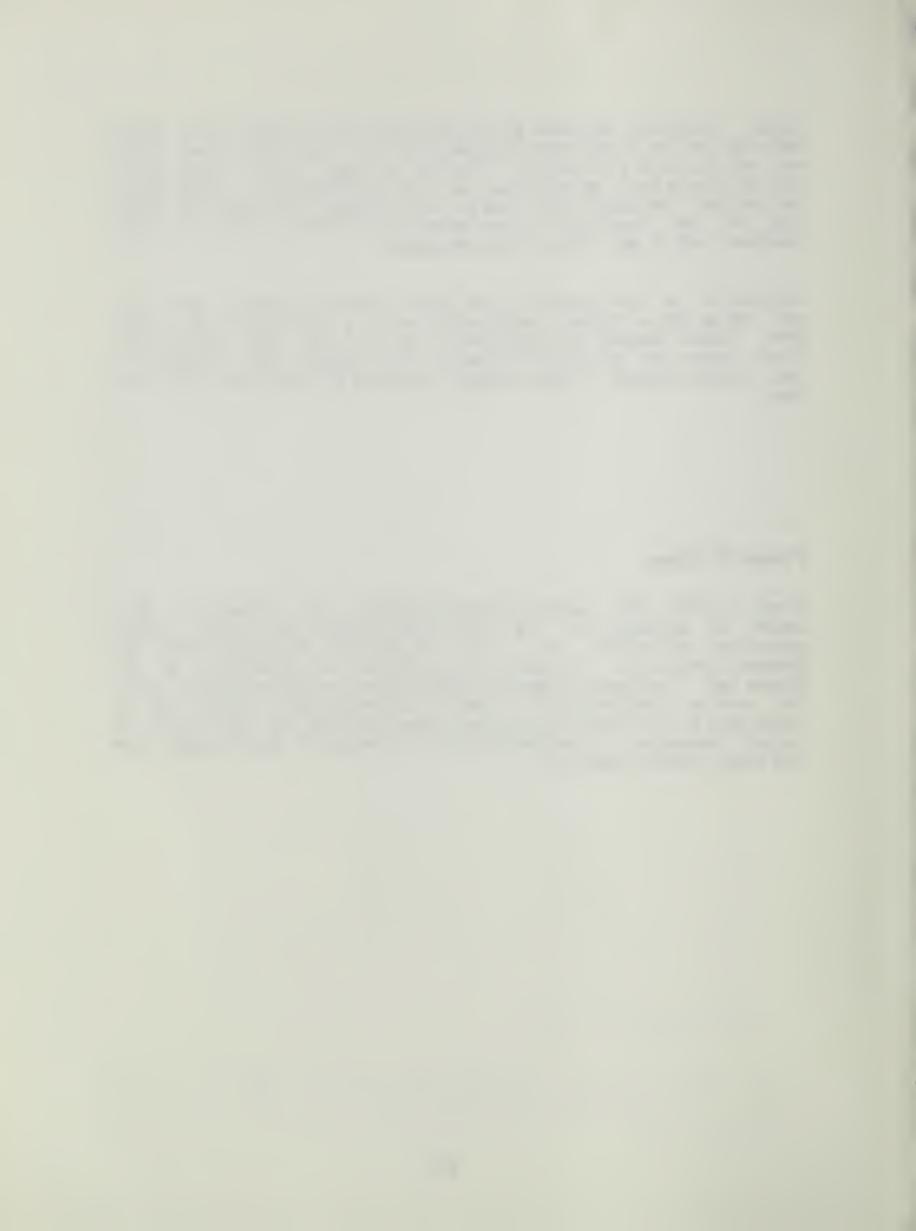


Actual dimensions measured on the ground may vary slightly from those measured on the flood hazard photomaps of this report due to map scale and reproduction limitations. Also due to scale, small raised areas such as houses built on earth pads will not be detectable. Originally, the 500 year frequency flood line was also planned to be shown on these maps but was found that in many areas the two lines were so close together that it was difficult to show both.

Information on the possibility of future floods of various magnitudes and the extent of flooding which might occur is included for the study area. Tables showing the elevations of the 10-, 50-, 100-, and 500-year flood events are included in Appendix C for selected cross sections of the various streams. Cross section locations are shown on individual maps.

Flood Profiles

Flood profiles for various storm frequencies are included in this report as appendix B. The flood profiles show the water surface elevations of the 10-, 50-, 100-, and 500-year frequency floods for present conditions. Included on the profiles are elevations of the stream bed, pertinent bridge and roadway data, and other location data. The profile stationing is in terms of stream distance in feet and is based upon high channel flow distances measured from the 1981 flight of aerial photomaps. Flood depths can be estimated at any location from the water surface profiles.



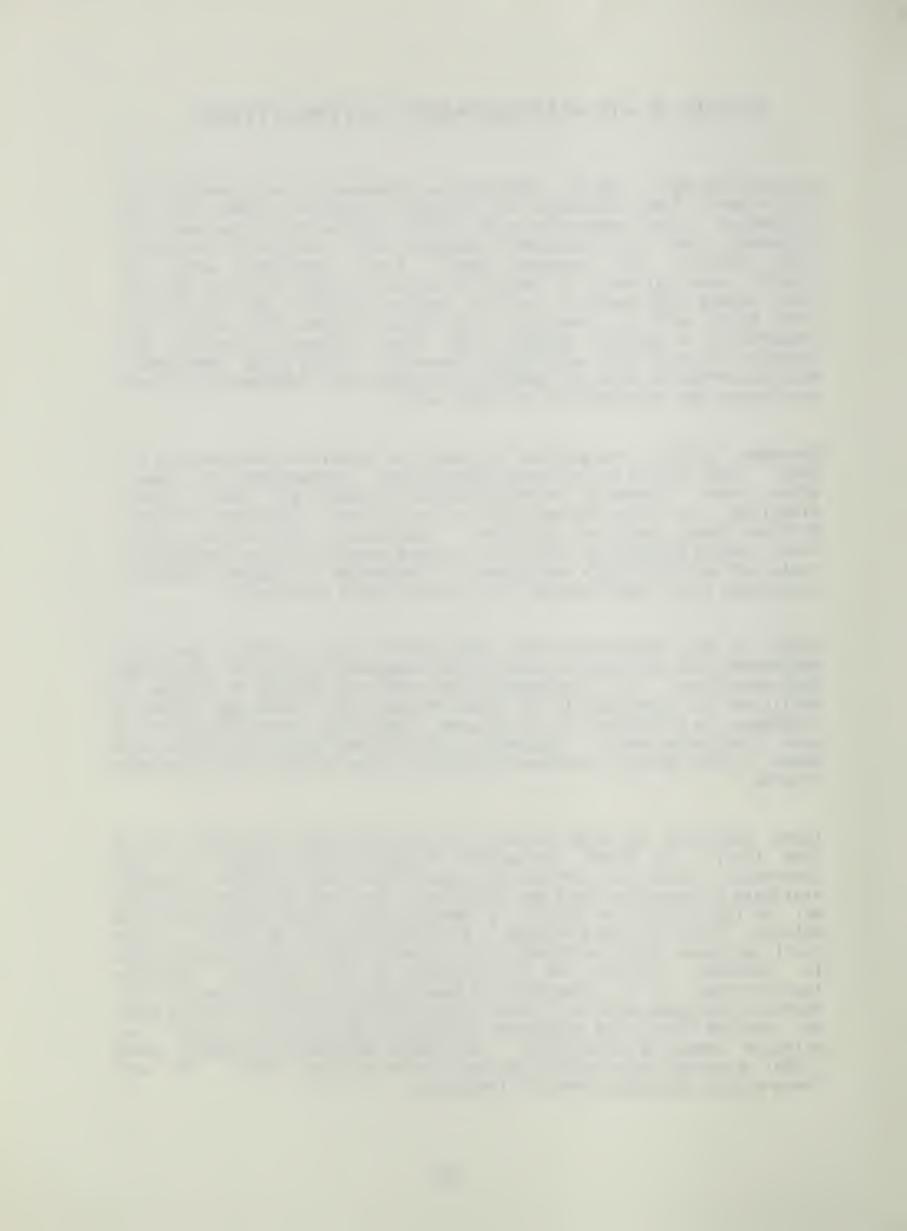
FLOOD PLAIN MANAGEMENT ALTERNATIVES

By using the maps, tables, and profiles presented in the appendices to this report, flood elevations at locations along the streams may be determined. This information will permit local units of government to implement flood plain management programs which recognize potential flood hazards. Such programs usually limit flood-prone areas to specific uses that would not result in serious economic loss nor loss of life during flood events. Building codes may preclude the flood plain from being used for housing, or it could require that houses be constructed a specific height above flood frequency elevation by building on earth pads or pilings. Generally, flood plain management must be worked out with the landowners involved with consideration given to alternatives available for the local area.

The maps, tables, and profiles are based on conditions that existed in 1983. Such factors as increased urbanization, encroachment on flood-prone areas, relocation or modification of bridges and other stream crossings, and stream channel modification can have significant effects on flood stages and areas inundated. Therefore, the results of any flood hazard evaluations should be reviewed periodically by appropriate state and local officials and planners to determine if changed watershed conditions would significantly affect future flood elevations.

Based on the flood plain areas identified in this report, the SCS recommends that an effective flood plain management program be implemented and maintained. It is recommended that the city develop a program to publicize the availability of flood insurance and encourage community residents to participate in the program, especially those located in or near flood-prone areas. Residents in flood-prone areas should be made aware of the impacts of non-participation in the National Flood Insurance Program.

Flood insurance was established by the National Flood Insurance Act of 1968 (Public Law 90-448, as amended) to make limited amounts of flood insurance, which was previously unavailable from private insurers, available to property owners and occupiers. The Flood Disaster Protection Act of 1973 (Public Law 93-234, as amended) was a major expansion of the National Flood Insurance Program. Flood Insurance is available through local insurance agents and brokers only after a city or county applies and is declared eligible for the program by the Federal Insurance Administrator, U. S. Department of Housing and Urban Development (HUD). Adoption and enforcement of a local flood prevention ordinance which meets HUD minimum flood plain management criteria is necessary to qualify and maintain community eligibility. The Federal Emergency Management Agency (FEMA) provides large scale flood maps for many urban areas. HUD uses these maps to determine rates of insurance.



In those communities participating in the HUD program, owners and occupiers of all buildings and mobile homes in the entire community are eligible to obtain flood insurance coverage. It is recommended that buildings and mobile homes within or adjacent to the delineated flood hazard areas carry flood insurance on the structure and contents.

The SCS can provide technical assistance through the Lee County Soil and Water Conservation District to Federal, State, and local agencies in the interpretation and use of the information contained herein and will provide additional technical assistance and data needed in local flood plain management programs upon request, as funding and personnel limitations permit.

Flood damage reduction can only be achieved through proper recognition of the hazards associated with flood plain development. Flood damages can be minimized by careful planning and proper flood plain management. Flood plain management programs should contain both preventive and corrective measures.

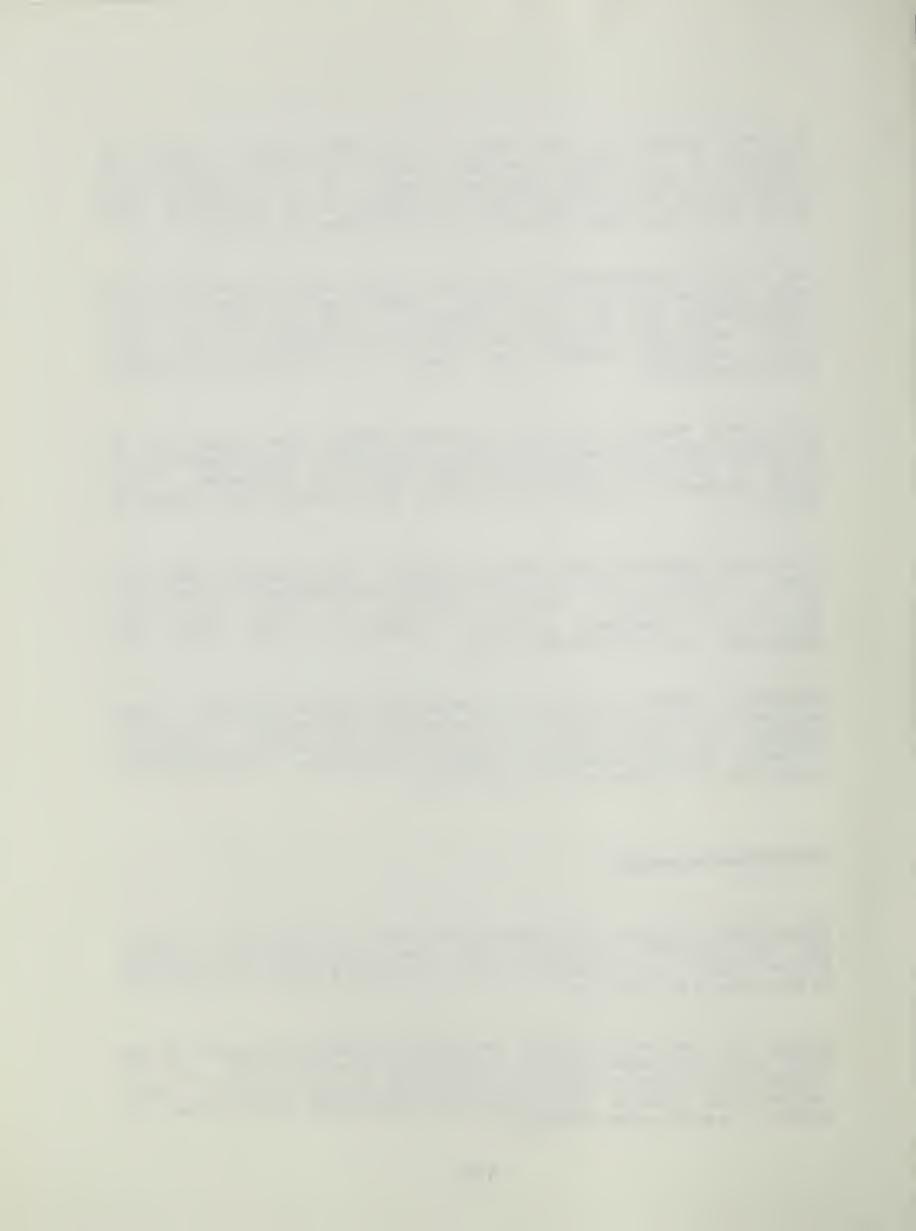
Preventive measures do not prevent flooding. These measures reduce the threat of damage or loss of life from flooding by regulating development in the flood plains. Preventive measures can include flood plain regulations, development policies, greenbelts or open spaces, tax adjustments, and flood warning systems.

Corrective measures also do not necessarily eliminate flooding. These measures can reduce the extent of flooding and flood damages. Corrective measures are usually physical measures and can include land treatment, floodwater retarding structures, channel rectification, floodproofing of structures, and evacuation of flooded areas.

Preventive Measures

Encroachment lines are the lateral boundaries of a designated floodway. They are definitely established lines, one on each side of the stream. Between these lines no construction or filling which causes an impediment to flow should be permitted.

Zoning is a legal method used to implement and enforce the details of the flood plain management program, to preserve property values, and to achieve the most appropriate and beneficial use of available land. Clear, concise, and thorough zoning bylaws with enforcement of the bylaws are essential to make zoning effective.



Subdivision regulations are used to specify the manner in which land may be subdivided. Regulations may state the required width of streets, requirements for curbs and gutters, size of lots, percentage of open space, and other points pertinent to the welfare of the community.

Building codes are developed to set up minimum standards for controlling the design, construction, and quality of materials used in buildings and structures within a given area to provide safety for life, health, property, and public welfare. Building codes can be used to minimize construction and subsequent damages resulting from inundation. Proper building restriction codes can specify adequate anchorage to prevent flotation of buildings from their foundations, prohibit storage of hazardous chemical or electrical equipment storage and establish minimum building foundation elevations.

Development policies are sound policies and decisions which are designed to prevent construction of streets and utility systems in flood prone areas. This tends to slow development of the flood plains.

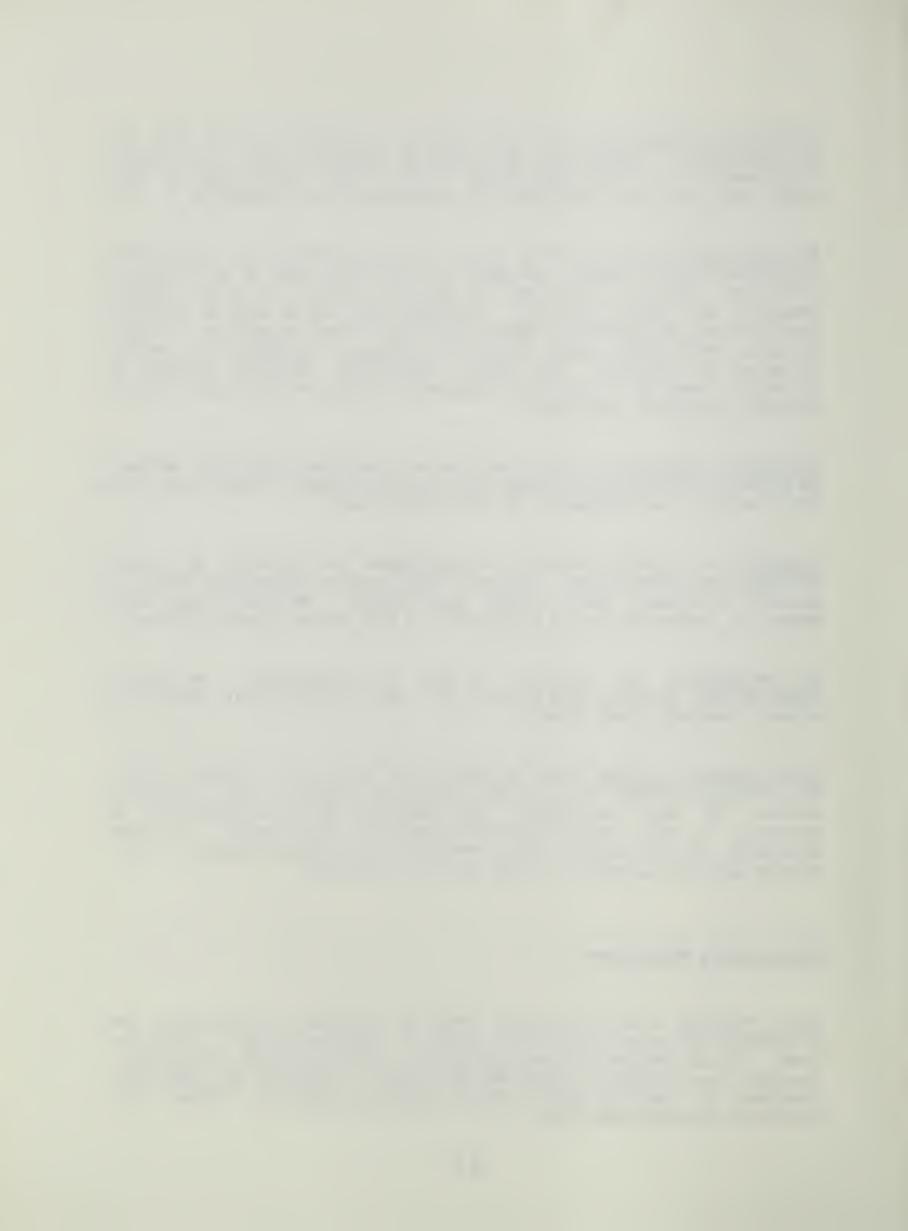
Greenbelt is a term related to the development and retention of stream frontages and flood plains. The use of these public and private lands for pasture or grazing, picnic areas, golf courses, and similar uses would materially reduce the damage potential in a high hazard flood plain area.

Tax adjustments for land that is used for agriculture, recreation, conservation, or other open space uses, may be effective in preserving natural floodways along streams.

Flood warning systems should be coordinated with local disaster plans. The National Weather Service issues warning of potential flood producing storms. On small watersheds, staff gages set at key locations can be monitored to give advance warnings upon flood prone areas. A float activated, battery powered signal connected to the local police or fire station would be desirable if high risks are involved.

Corrective Measures

Land treatment practices modify floods by increasing infiltration and decreasing the amount and rate of runoff. Practices include vegetative cover, runoff interceptors and diversions, erosion control structures, terraces, and cropping management practices. They can be especially important in reducing erosion and the resulting amount of sediment and pollutants carried downstream.



Floodwater retarding structures are earthfill or concrete impoundments to check the uncontrolled flow of floodwater. These structures are usually located to intercept the water from large drainage areas thus providing the maximum amount of downstream protection possible. Retarding structures may include dug pits in areas where ground water tables are well below the ground surface. Such pits require that stored water be pumped out following each storm event.

Channel alteration may be considered to improve the flow characteristics of the channel to enable it to safely pass the design flood. Such improvement is usually accomplished by enlarging, straightening, and/or lining the channel with due regard to minimizing the effect on surrounding natural environment. Channel work is likely to be cost prohibitive for the study area.

Maintain culverts and road ditches so that they are free of debris and allow water to flow freely.



Figure 6. Culvert filled with sediment and vegetation on L-1 (Trib to Yellow Fever Creek). This culvert will be ineffective and street flooding will result.

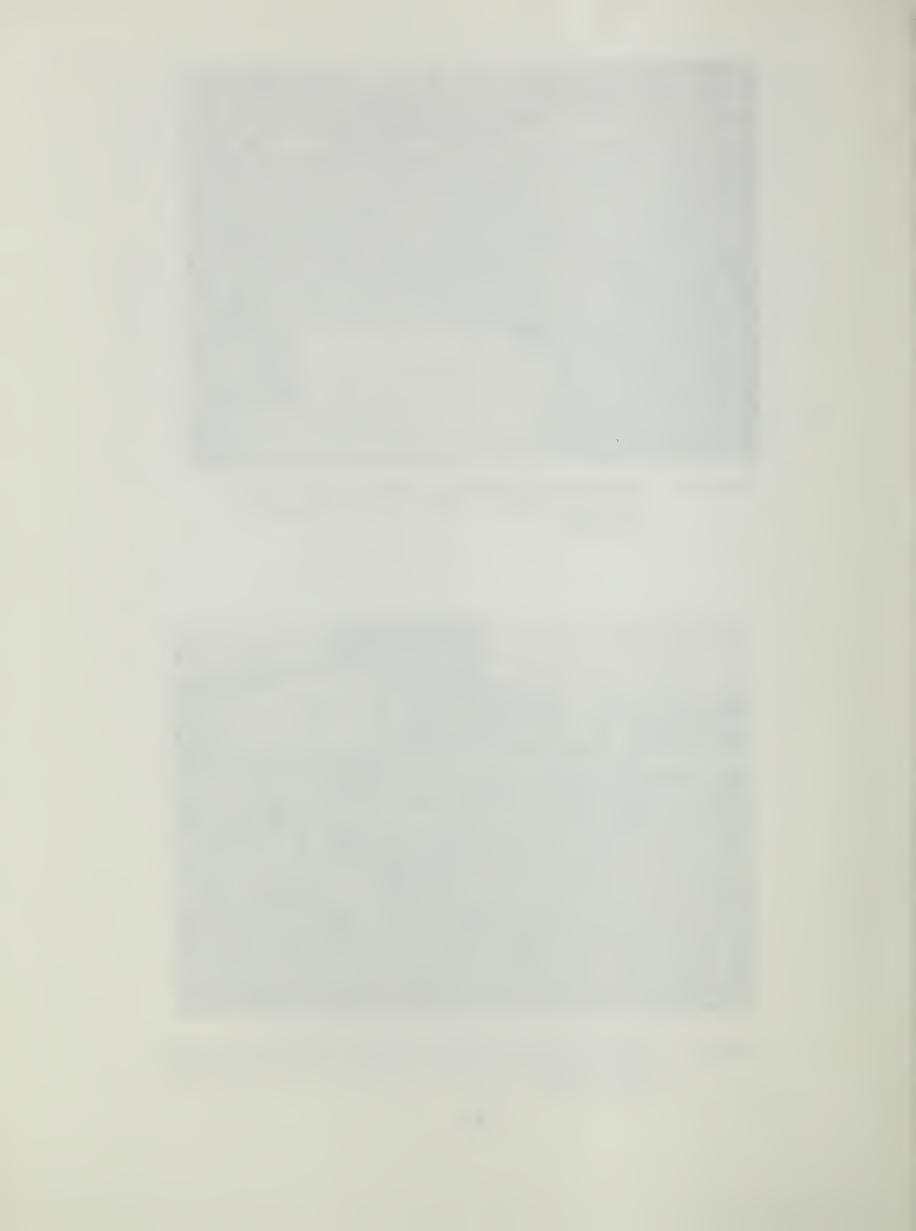




Figure 7. Improved road crossing of Marsh Point Creek at Bayshore Road. View is of upstream side of bridge.



Figure 8. Typical culvert system on Powell Creek Trib (U.S.41) Canal during dry season. View is of the downstream side of road.



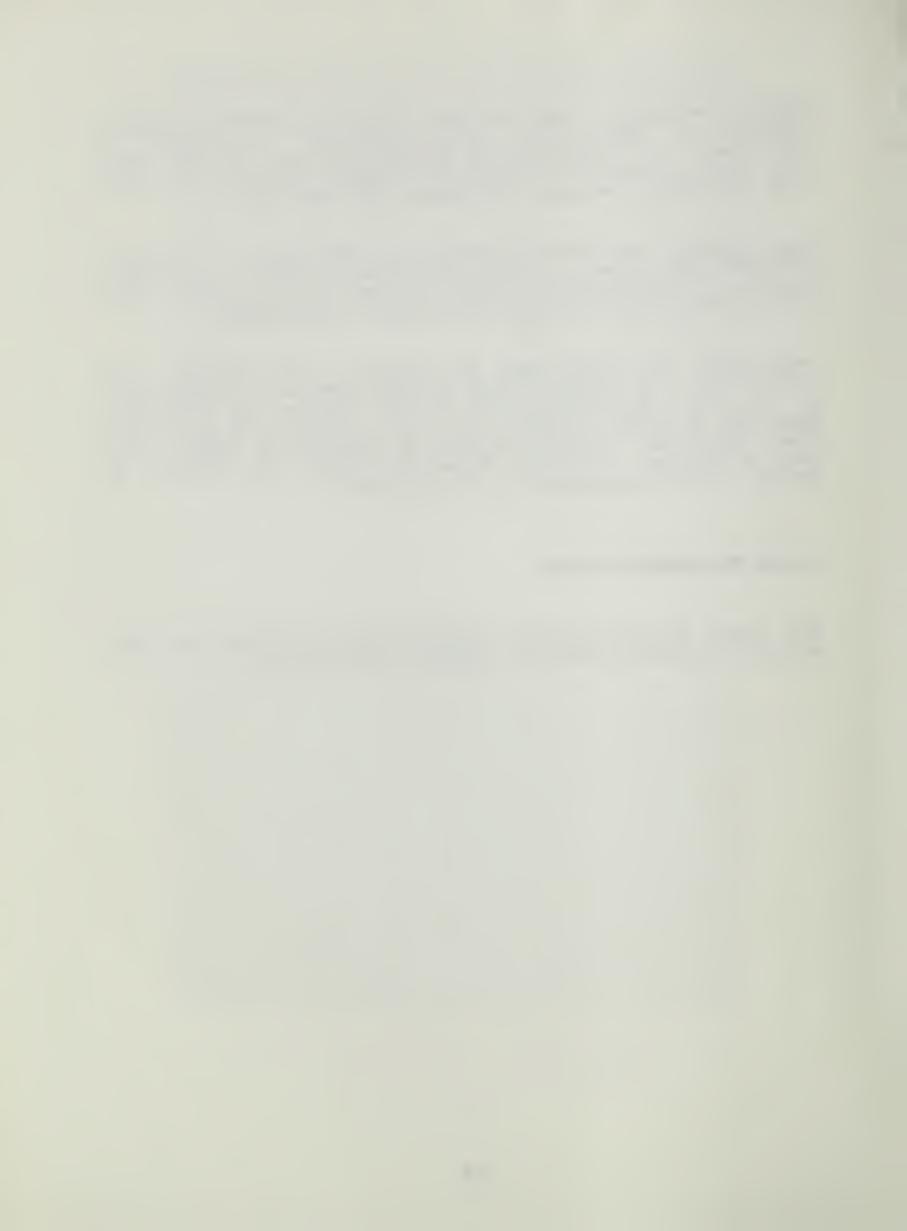
Permanent evacuation of developed areas subject to inundation usually involves the acquisition of lands by purchase, the removal of improvements, and the relocation of the population from such areas. Such lands could be used for parks and other purposes that would not suffer large flood damages and would not interfere with flood flows.

Flood proofing can reduce flood damages by a combination of structural provisions and changes or adjustments to properties subject to flooding. Examples of flood proofing are sealing low window and door openings, and modifying floor drains to prevent the entrance of flood waters.

Combinations of various types of practices, both structural and nonstructural, can normally provide a higher degree of flood protection, at less cost, than most individual types of practices by themselves, especially in highly developed flood plains similar to the Lee County flood hazard area. Careful intermixing of the most cost effective and socially acceptable individual measures can enhance the potential to provide a socially acceptable level of protection.

Local Recommendations

This report should be adequately publicized for its findings to be made available to property owners and occupiers in the study area.



GLOSSARY OF TERMS

Bridge Area -- The effective hydraulic flow area of a bridge opening accounting for the presence of piers, attached conduits, and skew (alignment), if applicable.

<u>Channel</u> -- A natural or artificial water course of perceptible extent with definite bed and banks to confine and conduct continuously or periodically flowing water.

Flood -- An overflow of water on lands not normally covered by water. Floods have two essential characteristics: the inundation of land is temporary; and the land is adjacent to and inundated by overflow from a river, stream, ocean, lake, or other body of standing water.

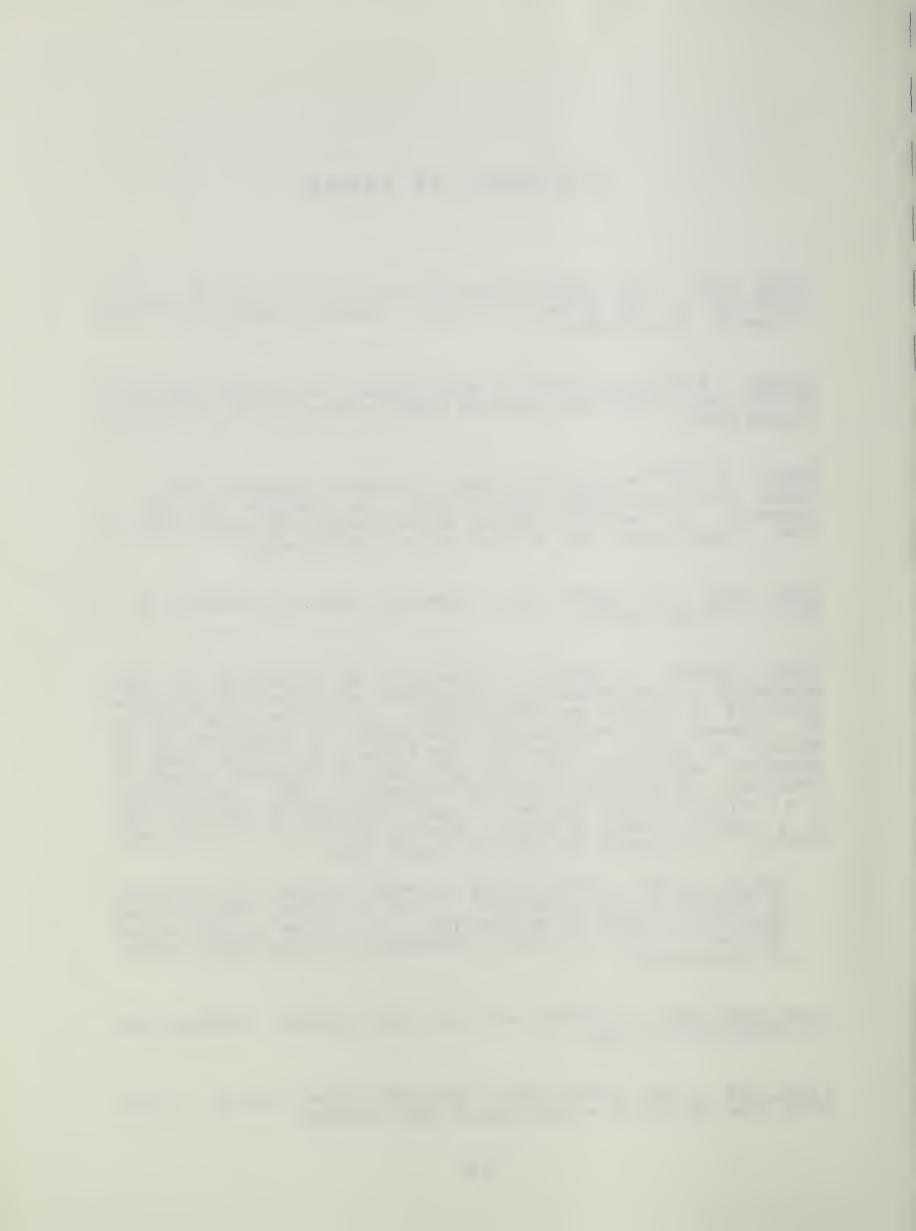
<u>Flood Crest</u> -- The maximum stage of elevation reached by the waters of a flood at a given location.

Flood Frequency -- A means of expressing the probability of flood occurrences as determined from a statistical analysis of representative streamflow or rainfall and runoff records. It is customary to estimate the frequency with which specific flood stages or discharges may be equalled or exceeded, rather than the frequency of an exact stage or discharge. Such estimates by strict definition are designated "exceedance frequence", but in practice the term "frequency" is used. The frequency of a particular stage or discharge is usually expressed as occurring once in a specified number of years. Also see definition of "recurrence interval." For example, see "100-year Flood" below:

100-year flood - a flood having an average frequency of occurrence in the order of once in 100 years. It has a l percent chance of being equalled or exceeded in any given year. It is based on statistical analyses of rainfall and runoff characteristics in the general region of the watershed.

Flood Hazard Area -- Synonymous with Flood Plain (general). Commonly used in reference to flood map.

Flood Peak -- The highest stage or discharge attained during a flood event; also referred to as peak stage or peak discharge.



Flood Plain (general) -- The relatively flat area or low lands adjoining the channel of a river, stream, or watercourse; ocean, lake, or other body of standing water which has been or may be covered by floodwater.

Flood Plain (specific) -- A definitive area within a flood plain (general) or flood-prone area known to have been inundated by a historical flood, or determined to be inundated by floodwater from a potential flood of a specific frequency.

Flood Prone Area -- Synonymous with Flood Plain (General)

<u>Flood Profile</u> -- A graph showing the relationship of water surface elevation to stream bed. It is generally drawn to show the water surface elevation for the peak of a specific flood, but may be prepared for conditions at a given time or stage.

Hydrologic Boundary - The divide separating adjoining watersheds

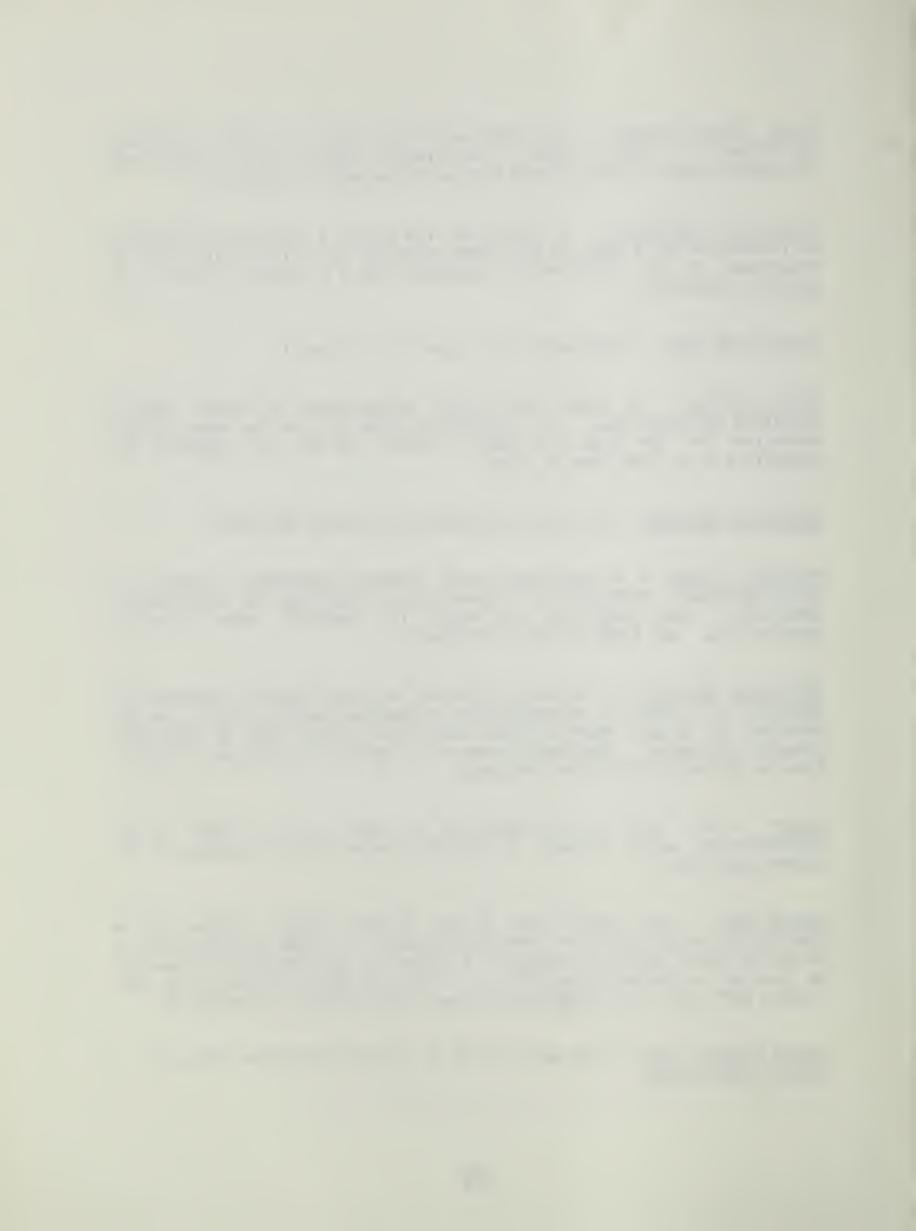
<u>Potential flood</u> -- A spontaneous event (natural phenomenon) capable of occurring from a combination of meteorological, hydrological, and physical conditions; the magnitude of which is dependent upon specific combinations. See Flood and Flood Frequency.

Recurrence Interval -- The average interval of time based on a statistical analysis of actual or representative streamflow records which can be expected to elapse between floods equal to or larger than a specified stage or discharge. Recurrence interval is generally expressed in years. Also see definition of Flood Frequency.

Runoff -- That part of precipitation as well as any other flood contributions, which appears in surface streams of either perennial or intermittent form.

Stream Bed -- The lowest part of the stream channel (either in a constructed cross section or a natural channel). Bottom elevations at a series of points along the length of a stream may be plotted and connected to provide a stream bottom profile. (This is often referred to as the "stream bed" and is so designated on the flood profiles in Appendix B).

Stream Channel Flow -- That water which is flowing within the limits of a defined watercourse.



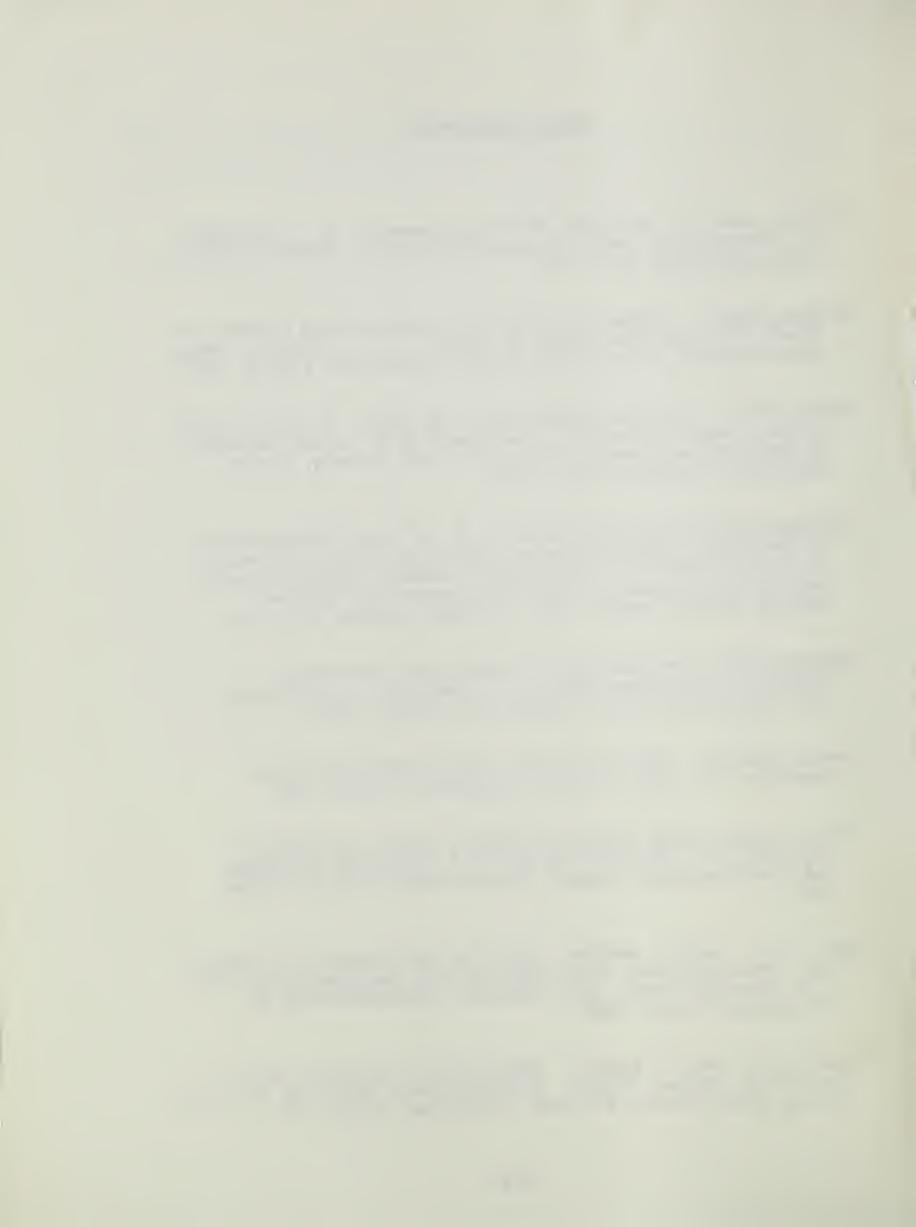
Structural Bottom of Opening -- The lowest point of a culvert or bridge opening with a constructed bottom through which a stream flows that could tend to limit the stream channel bottom to that specific elevation. This structural bottom may be covered with sediment or debris which further restricts the size of the opening.

<u>Watershed</u> -- A drainage basin or area which collects and transmits runoff usually by means of streams and tributaries to the outlet of the basin.



BIBLIOGRAPHY

- Bradley, Joseph N. Hydraulics of Bridge Waterways. U. S. Department of Transportation, Federal Highway Administration, Bureau of Public Roads, Washington, D.C., 1970.
- Bridges, Wayne C. Technique for Estimating Magnitude and Frequency of Floods on Natural-Flow Streams in Florida. U. S. Geological Survey, Water Resources Investigations 82-4012, Tallahassee, Florida, 1982.
- Brown, Mark T. The South Florida Study, Lee County: An Area of Rapid Growth. Center for Wetlands, the University of Florida and Bureau of Comprehensive Planning, Division of State Planning, Florida Department of Administration, 1979.
- "Climatography of the United States No. 81 (By State) Monthly Normals of Temperature, Precipitation and Heating and Cooling Degree Days 1941-70, Florida," U. S. Department of Commerce, National Oceanic and Atmospheric Administration, Environmental Data Service, National Climatic Center, Asheville, North Carolina, Aug. 1973.
- "Daughtrey-Trout Creek Watershed, Charlotte and Lee Counties, Florida," Investigation Report, U. S. Department of Agriculture, Soil Conservation Service, Gainesville, Florida, 1973.
- Fernald, Edward A. Atlas of Florida. Florida State University Foundation Inc., Rose Printing, Tallahassee, Florida, 1981.
- "Flood Insurance Study, City of Fort Myers, Florida, Lee County,"
 Tetra Tech, Inc., Contract # H-4059, U.S. Department of Housing
 and Urban Development, Federal Insurance Administration, October
 1978.
- Lopez, M.A. and W.M. Woodham. Magnitude and Frequency of Flooding on Small Urban Watersheds in the Tampa Bay Area, West-Central Florida, U. S. Geological Survey, Water Resources Investigations 82-42, Tallahassee, Florida, 1983.
- Missimer, T.M. and D.H. Boggess. Fluctuations of the Water Table in Lee County, Florida, 1969-73. Open-file Report 74019, U.S. Geological Survey, Water Resources Division, Tallahassee, Florida 1974.





RECORD ADDED

OCLC: 16906776 Ord#: 170993TE/6 Entrd: 871028 Used: 871028
Type: a Bib lvl: m Freq: Lang: eng Class: 1/gi/u Stat: receiv
Repr: Enc lvl: O Mtrl: Ctry: xx Forms: 0,0 Plan:

1 040 c AGL

2 245 00 Flood plain management study Lee County, Florida: b Powell, Daughtrey, Popash, Stroud, Marsh Point, Chapel Branch, Bayshore, and Thompson Cutoff Creeks and tributaries to Yellow Fever Creek / c prepared by U.S. Department of Agriculture, Soil Conservation Service, Gainesville, Florida; in cooperation with The Florida Department of Community Affairs and Lee County Soil and Water Conservation District and Lee County Commission.

3 260 O Gainesville FL: b U.S. Department of Asriculture, Soil Conservation Service, c 1984.

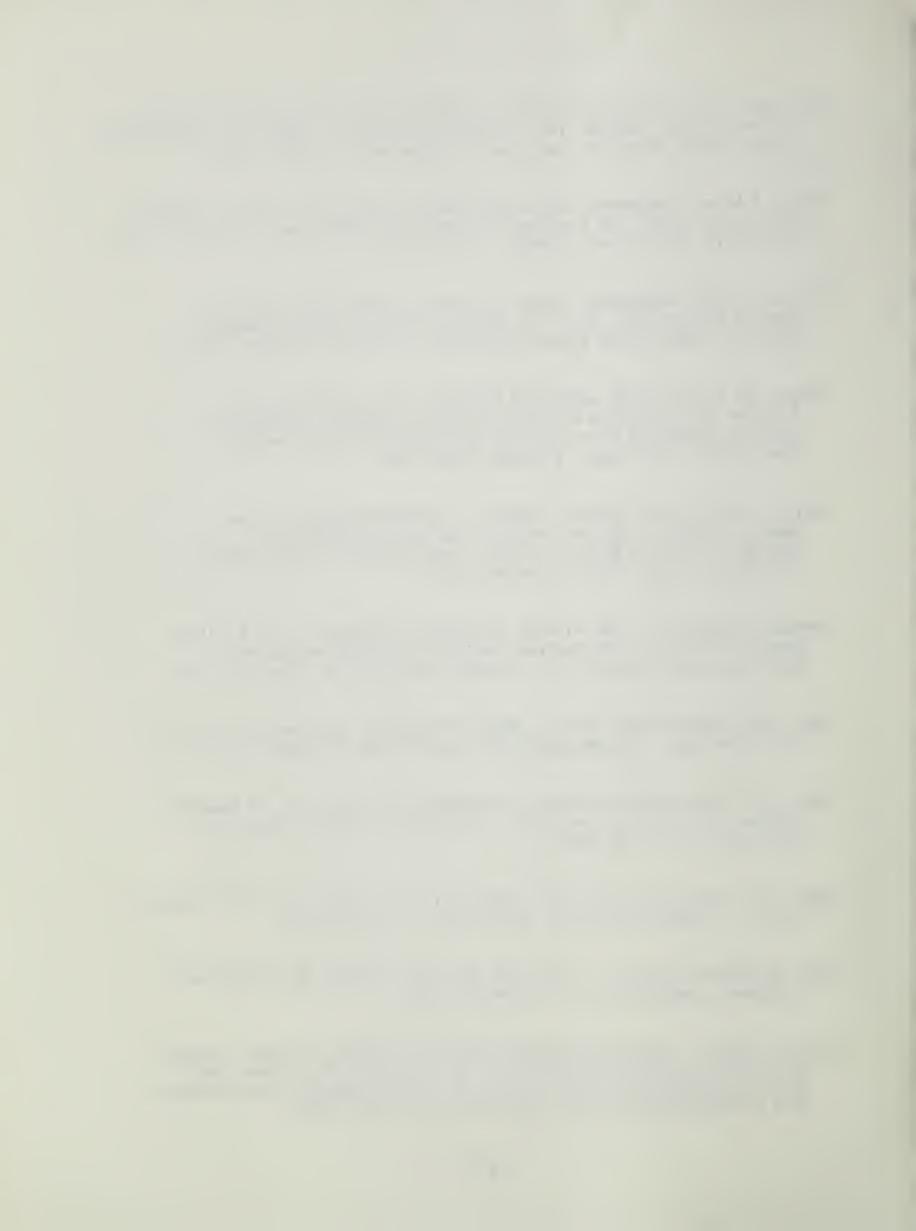
- 4 740 01 Lee County, Florida: flood plain management study.
- 5 SOURCE 170/US Nat Asr Libr
- 6 DESTIN ord c 1/ord d 1
- 7 ORDER 871028

Screen 2 of 2 -8 REMARKS "2" GIFT

RWW/VCP

THE RESERVE OF THE PROPERTY OF THE PERSON NAMED IN COLUMN TWO IN COLUMN TO THE PERSON NAMED IN C nonce the control of The service - North B. Asia THE PROPERTY OF THE PARTY OF TH

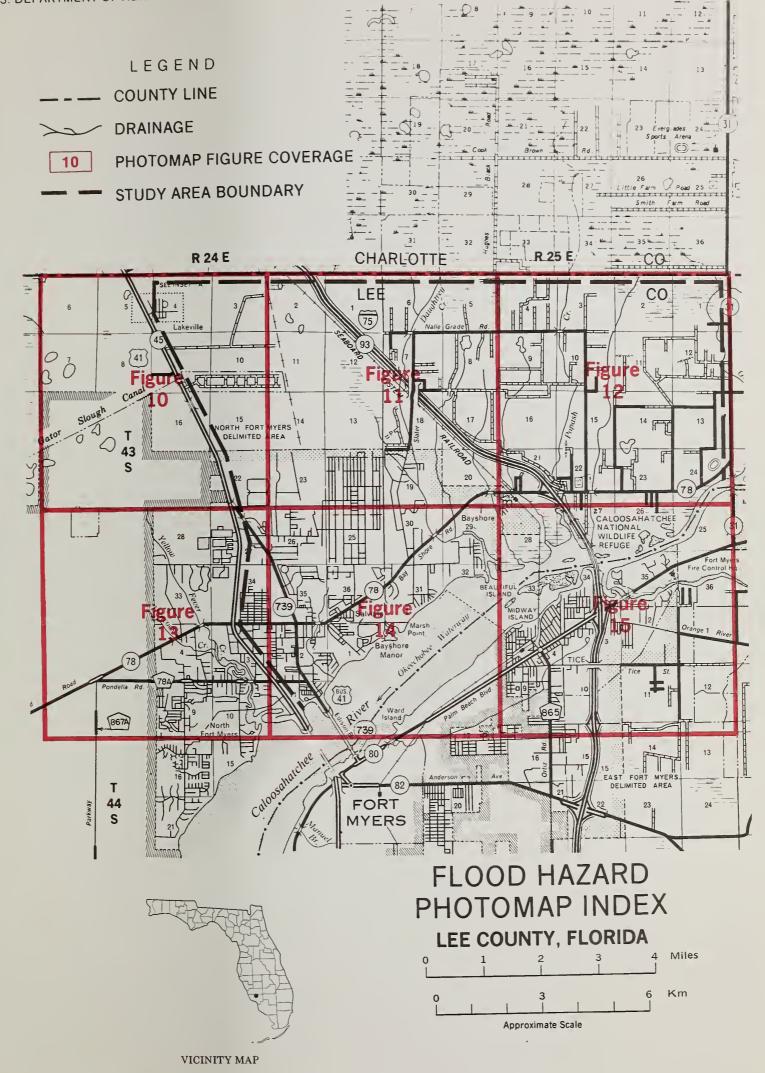
- Missimer, T.M. and T.H. O'Donnell. Fluctuations of Ground-Water Levels in Lee County, Florida, 1974. Open-File Report FL-75008, U.S. Geological Survey, Water Resources Division, Tallahassee, Florida, 1975.
- "Phase I Water Management Study, West Central Charlotte County," Johnson Engineering, Inc., for Southwest Florida Water Management District, Peace River Board, Fort Myers, Florida, July 1976.
- "Phase II Water Management Study, West Central Charlotte County,"
 Johnson Engineering, Inc. for Southwest Florida Water Management
 District, Peace River Board, Fort Myers, Florida, February 1977.
- "Phase III Gator Slough Watershed Analysis and Preliminary Plan -A Report for Charlotte and Lee Counties," Johnson Engineering, Inc. for Southwest Florida Water Management District, Peace River Board, Fort Myers, Florida, April 1983.
- Smally, Wellford and Nalven. Report on Water Management in Lee County, Florida. Smally, Wellford and Walven Consulting Engineers, for the Board of County Commissioners of Lee County, Florida, Sarasota, Florida, August 1961.
- Terhune, Frances W. 1983 Florida Statistical Abstract, 17th Edition. Bureau of Economic and Business Research, College of Business Administration, University of Florida, Gainesville, Florida, 1983.
- USDA, Soil Conservation Service, National Engineering Handbook Section Four-Hydrology. Engineering Staff, Washington, DC, August 1972.
- USDA, Soil Conservation Service, Technical Release No. 20, "A Computer Program for Project Formulation Hydrology", Engineering Staff Washington, D.C., May 1982.
- USDA, Soil Conservation Service, Technical Release No. 61, "WSP2 Computer Program, Engineering Staff, Washington, D.C. May 1976.
- Water Resources Council, A Unified National Program for Flood Plain Management, Washington, DC, September 1979.
- Woodfin, James. Rainfall Frequency Atlas of Alabama, Florida, Georgia, and South Carolina for Durations from 30 Minutes to 24 Hours and Return Periods from 1 100 Years. U.S. Department of Agriculture, Soil Conservation Service, Gainesville, Florida, 1978.



APPENDIX A

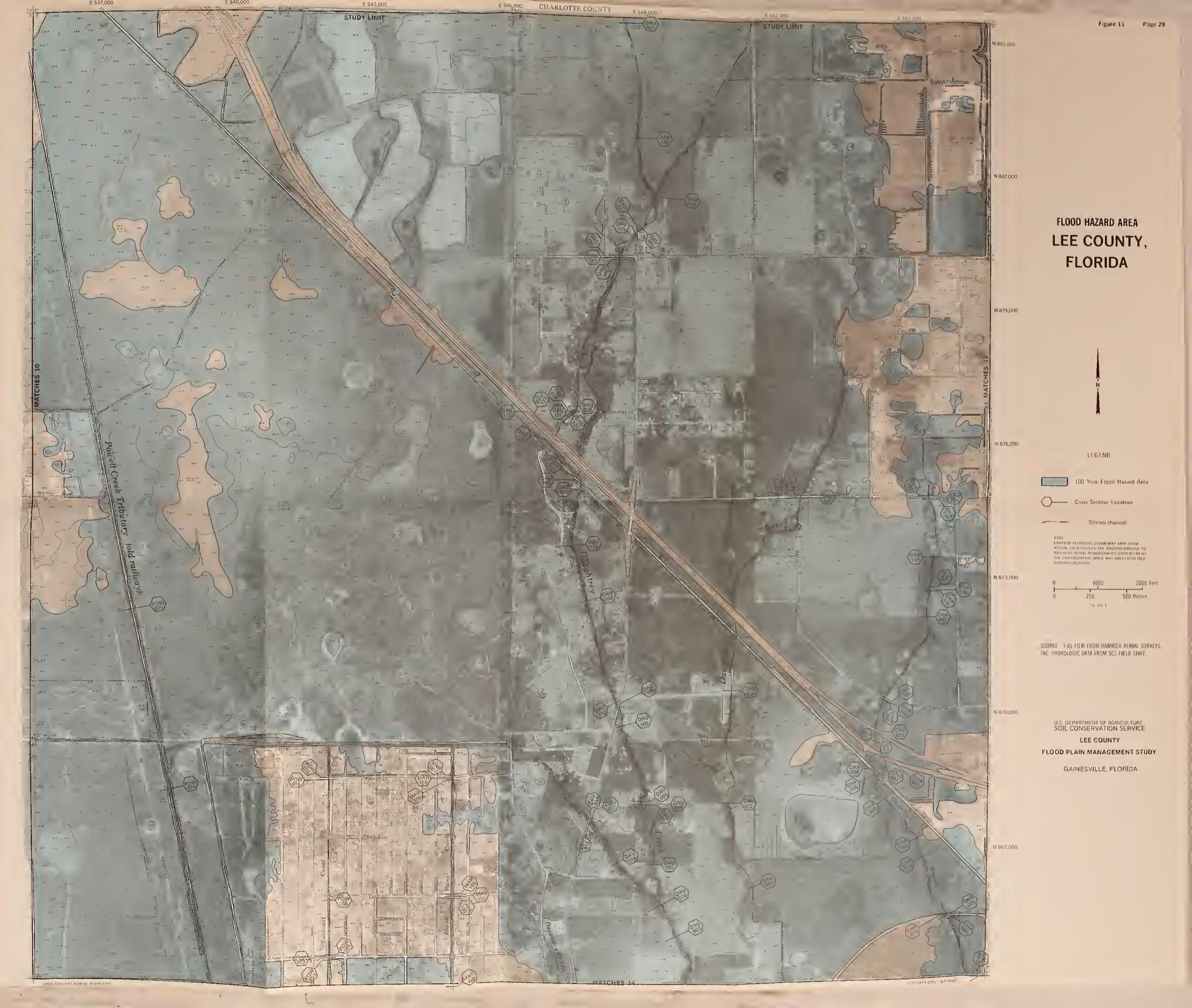
FLOOD HAZARD PHOTOMAPS



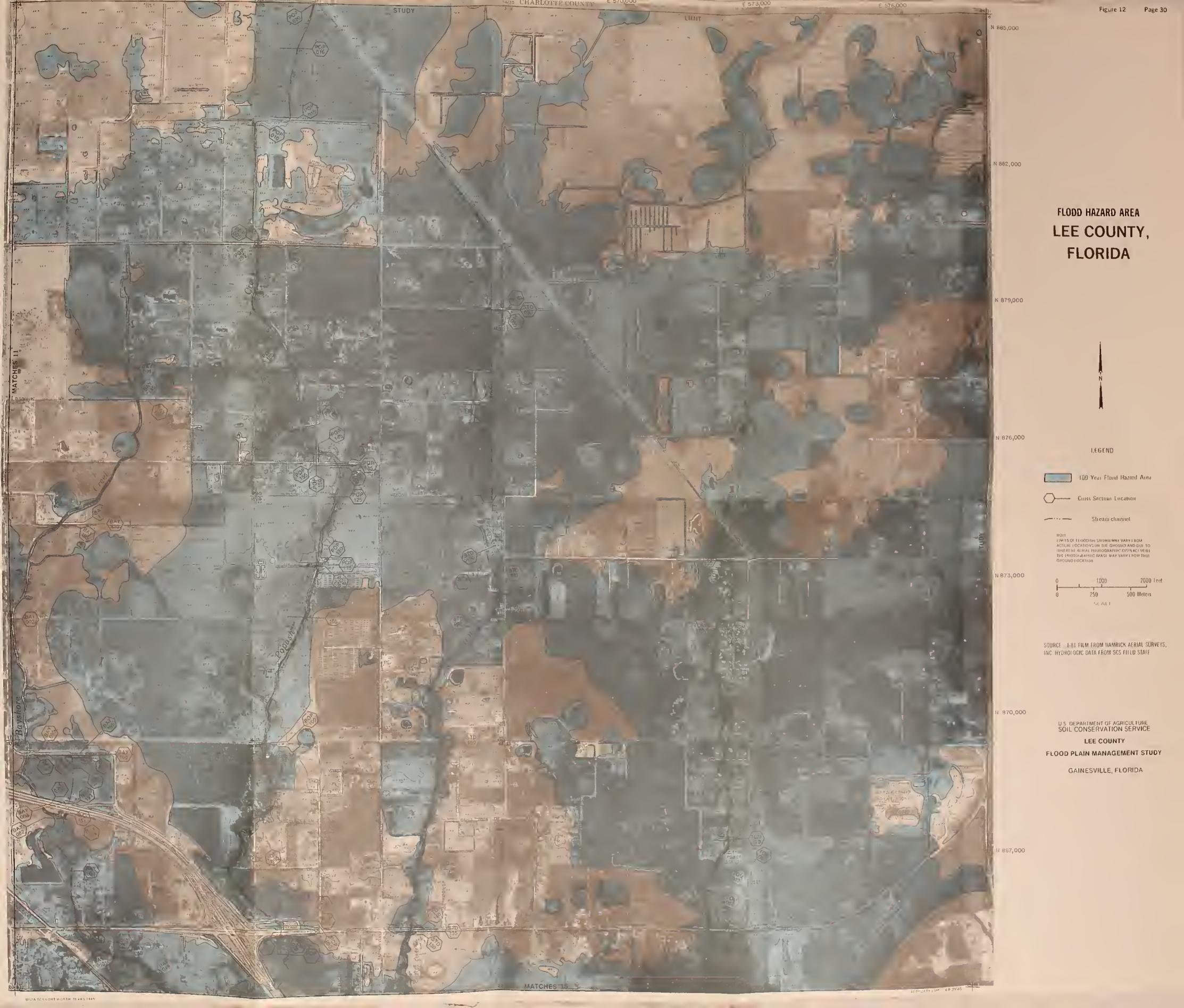




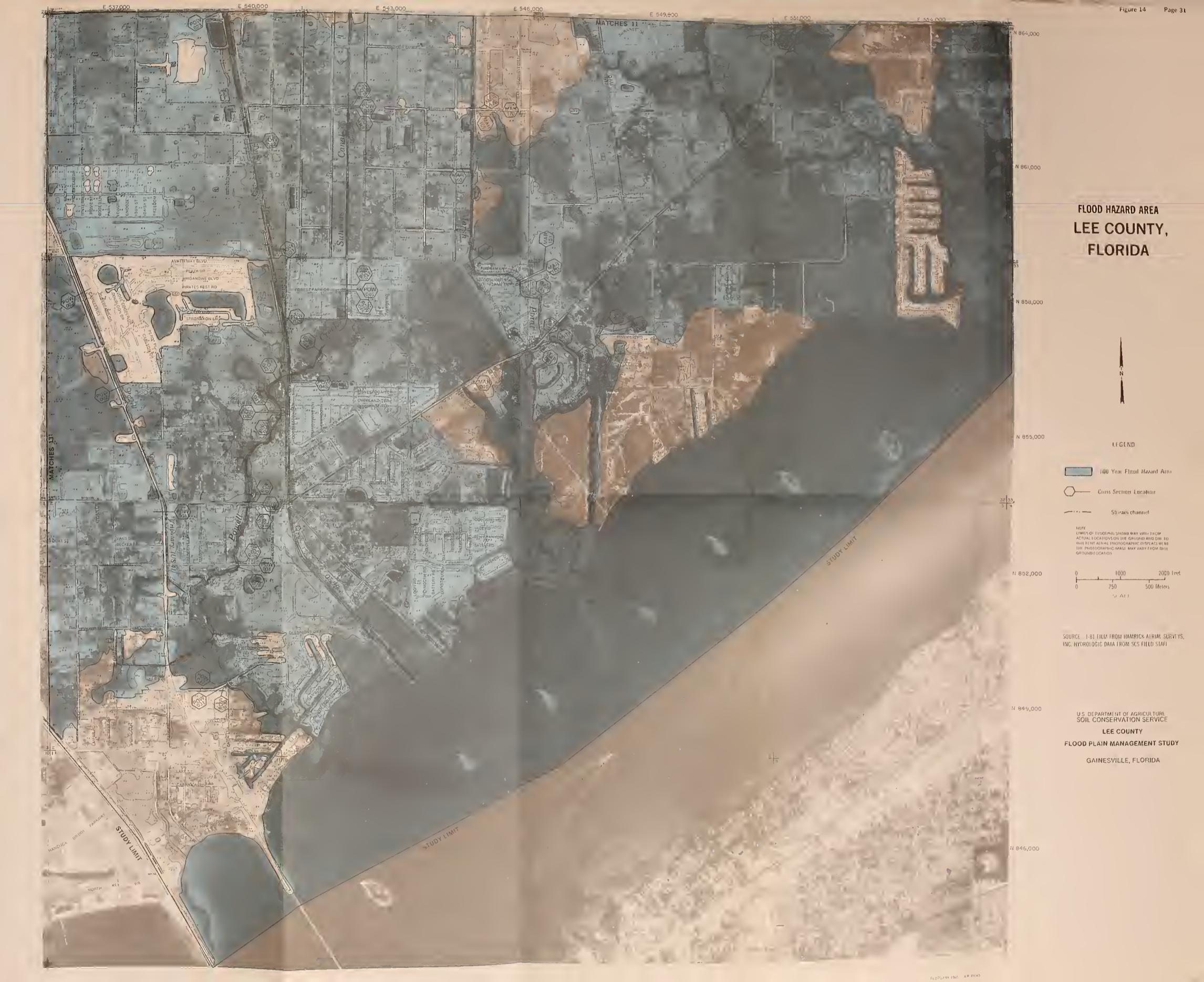
















FLOOD HAZARD AREA LEE COUNTY, **FLORIDA**

LEGEND

100 Year Flood Hazard Area

Cross Spection Location

Stream channel

NOTE
FIRSTS OF TROODING SHOWH MAY VARY FROM
ACTUAL COCATIONS ON THE GROUND ARD DUE TO
INDERTHE ACTION FROM PROPOSERABLE, DISPERCE MERE
THE 1990 COCATION
GROUND FOCATION

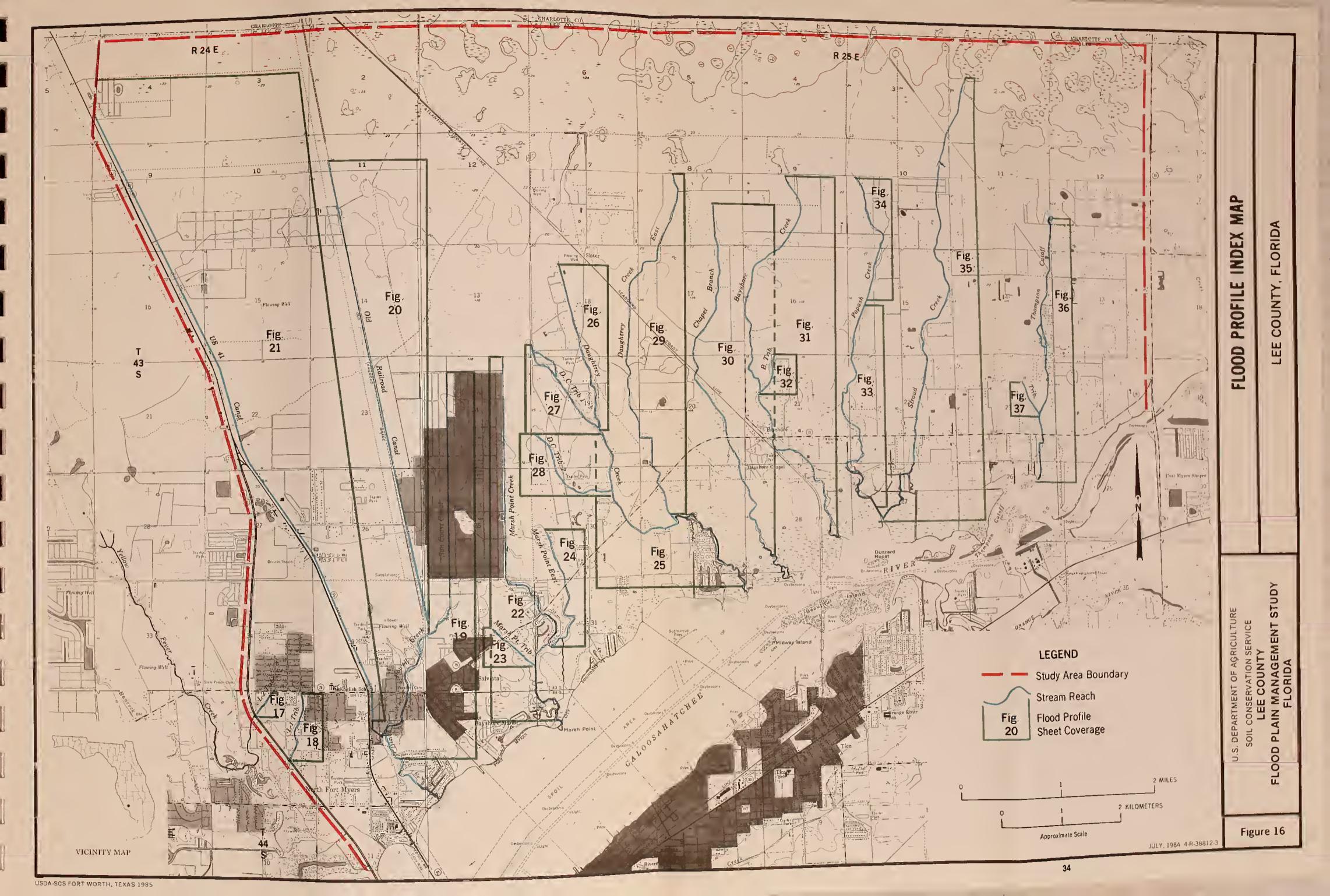
500 Meters

SOURCE: 1-81 FILIN FROM HAMRICK AERIAL SURVEYS. INC. HYDROLOGIC DATA FROM SCS FIELD STAFF

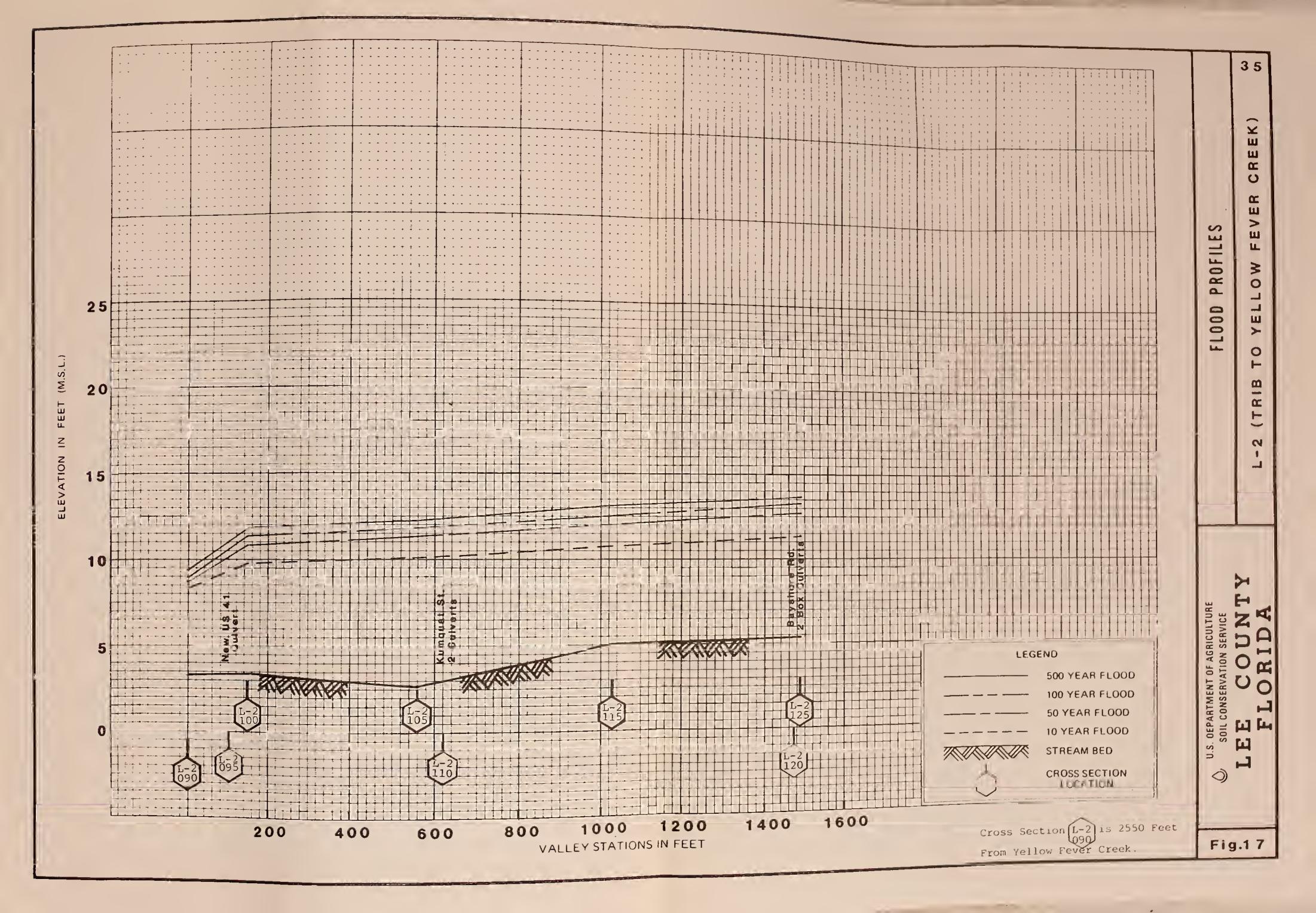
U.S DEPARTMENT OF AGRICULTURE SOIL CONSERVATION SERVICE LEE COUNTY FLOOD PLAIN MANAGEMENT STUDY

GAINESVILLE, FLORIDA

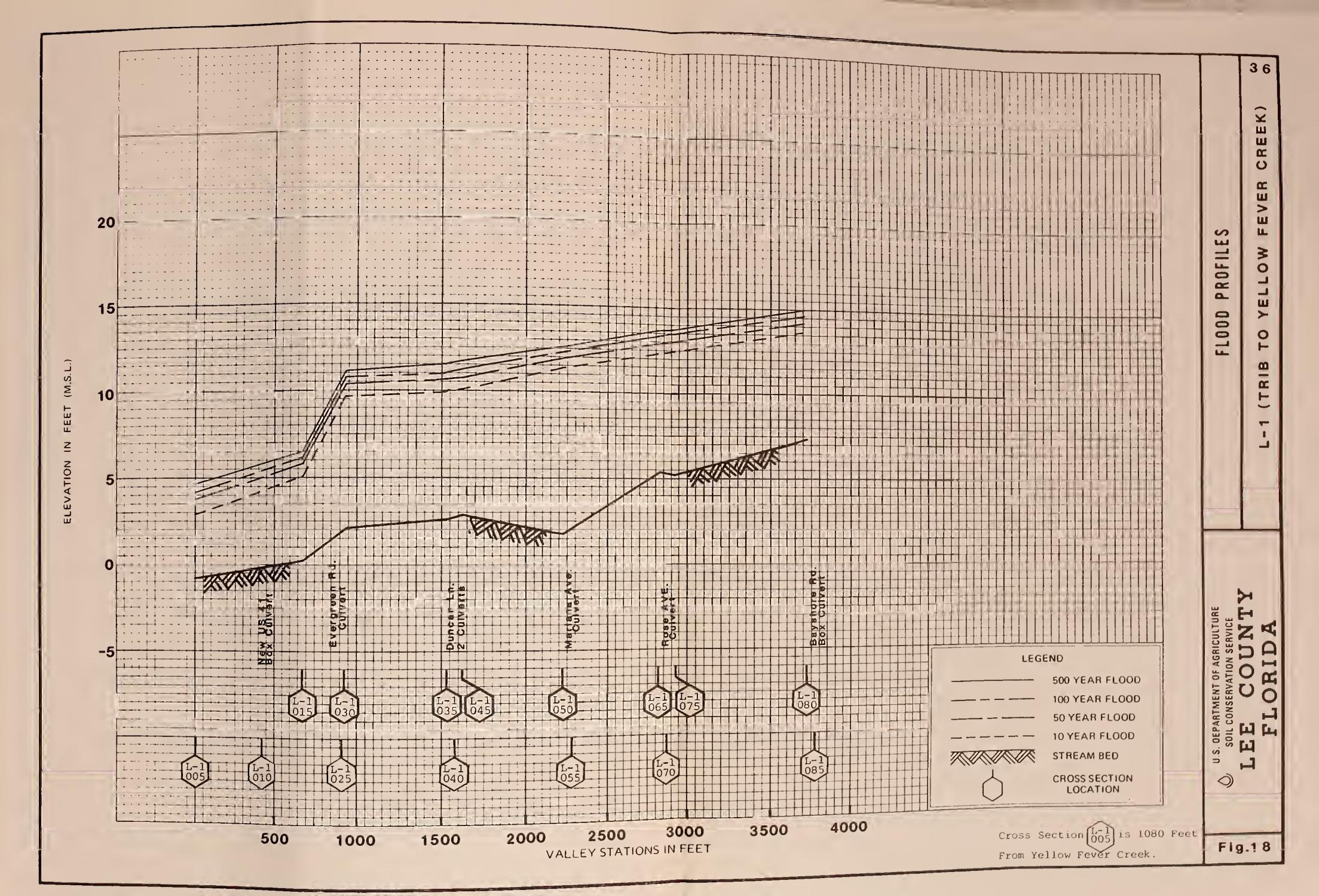




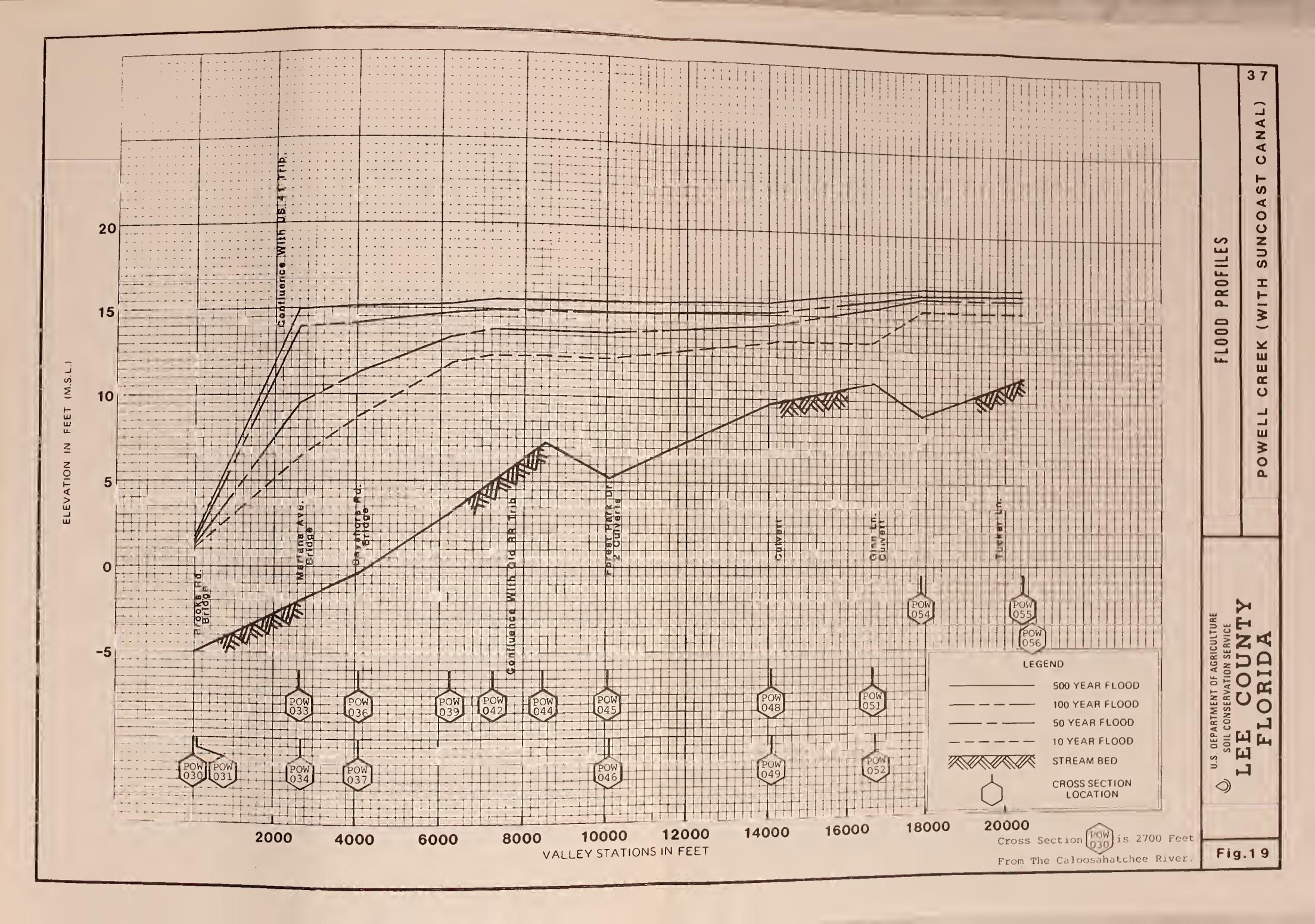




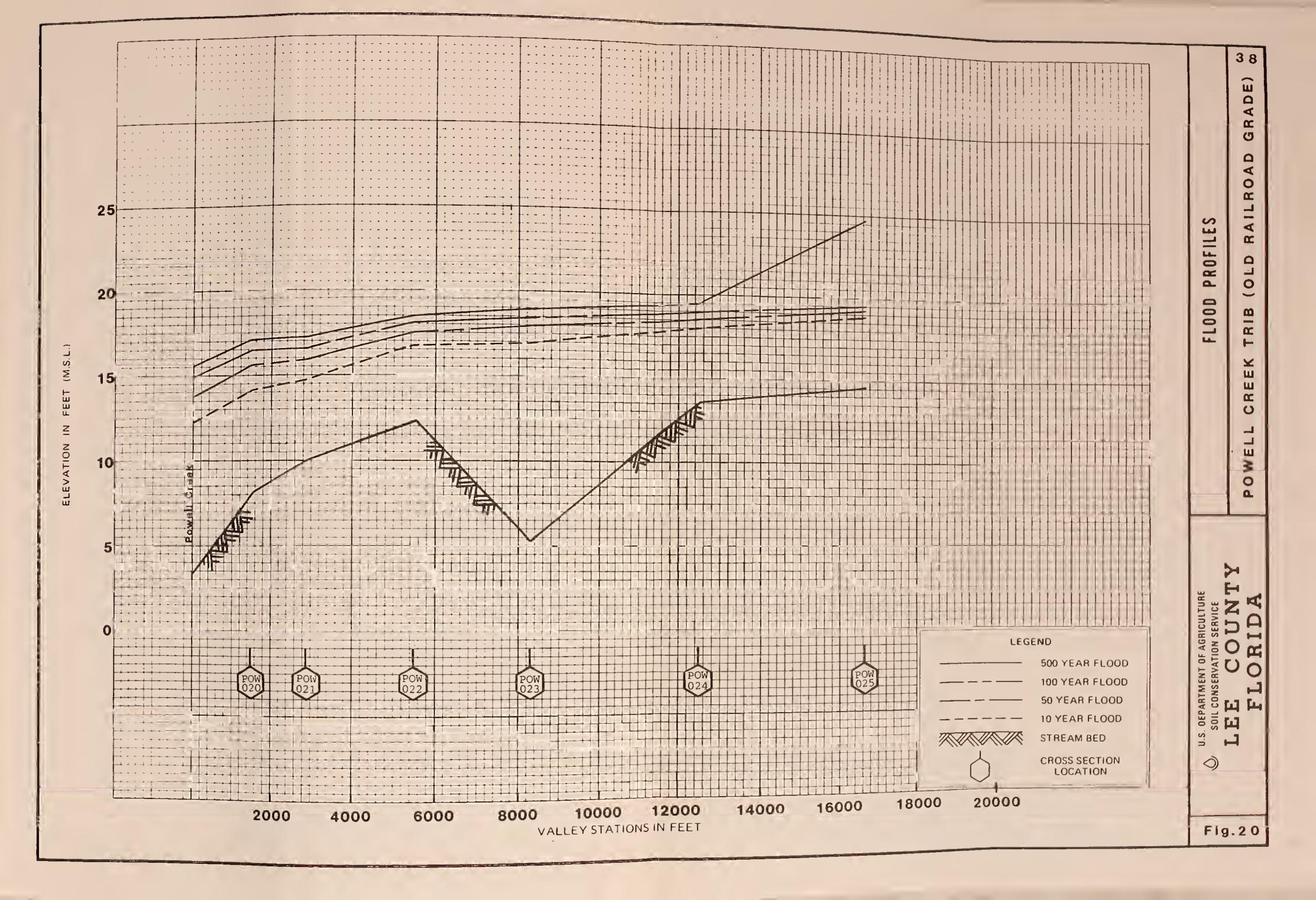




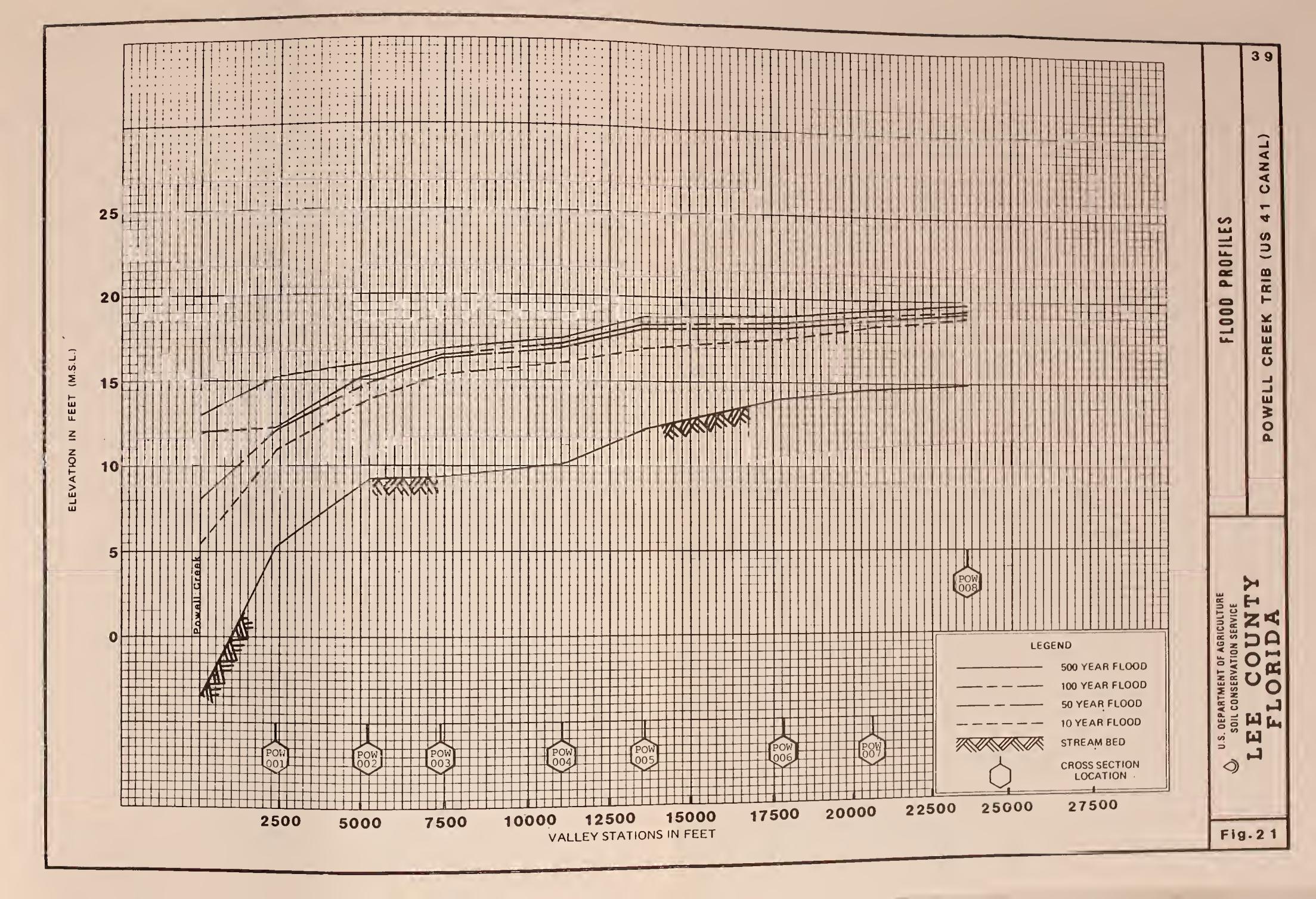




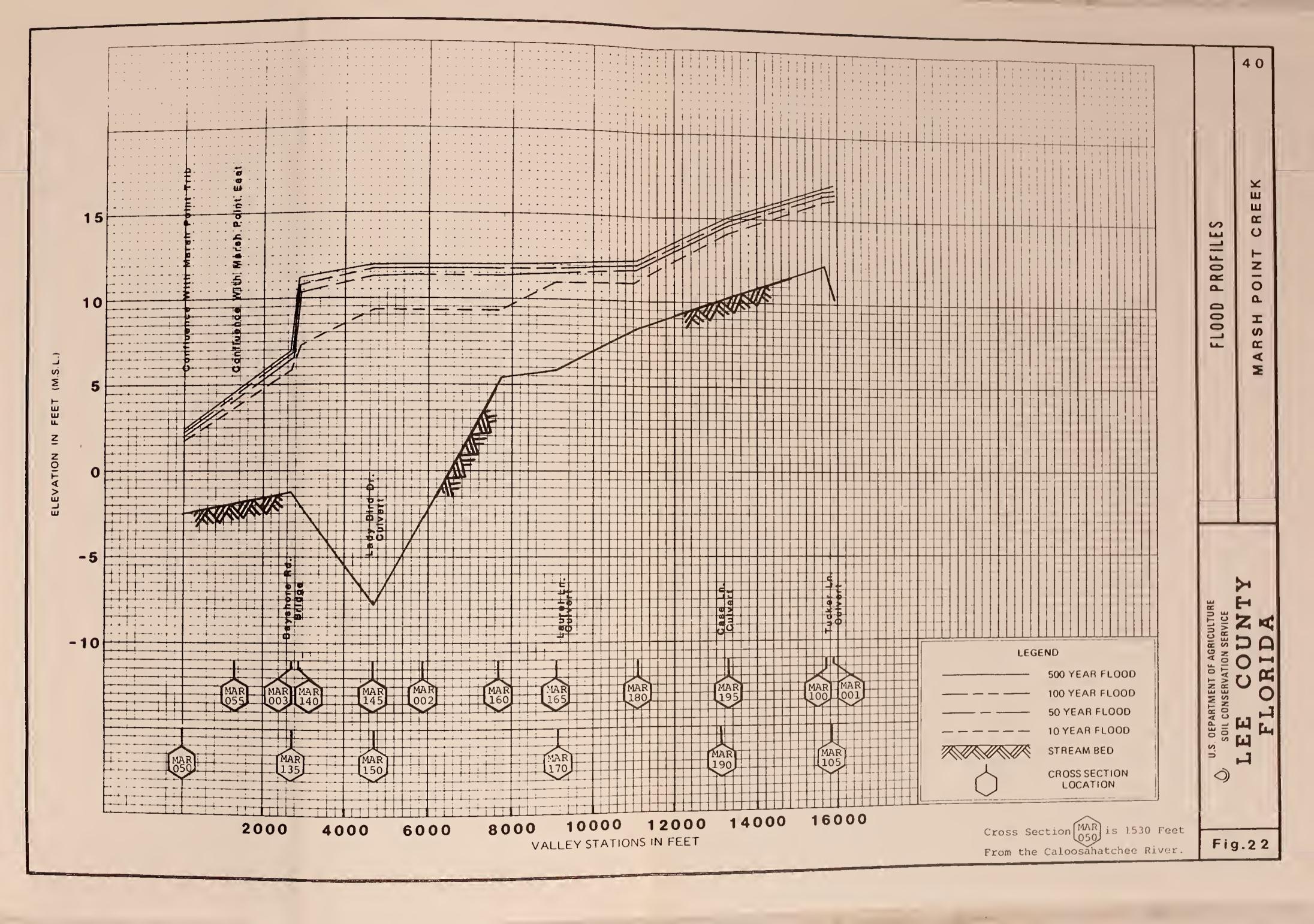




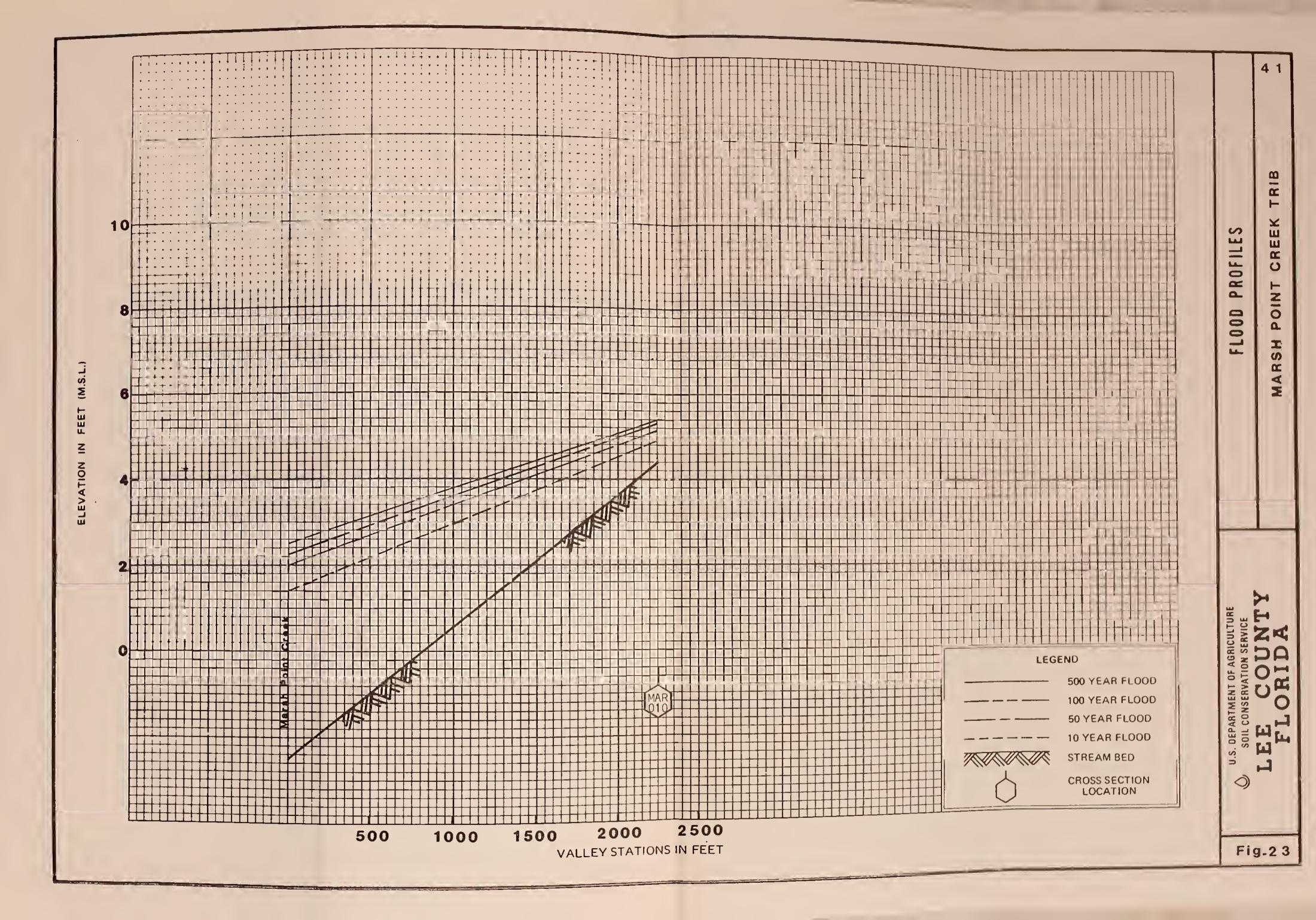




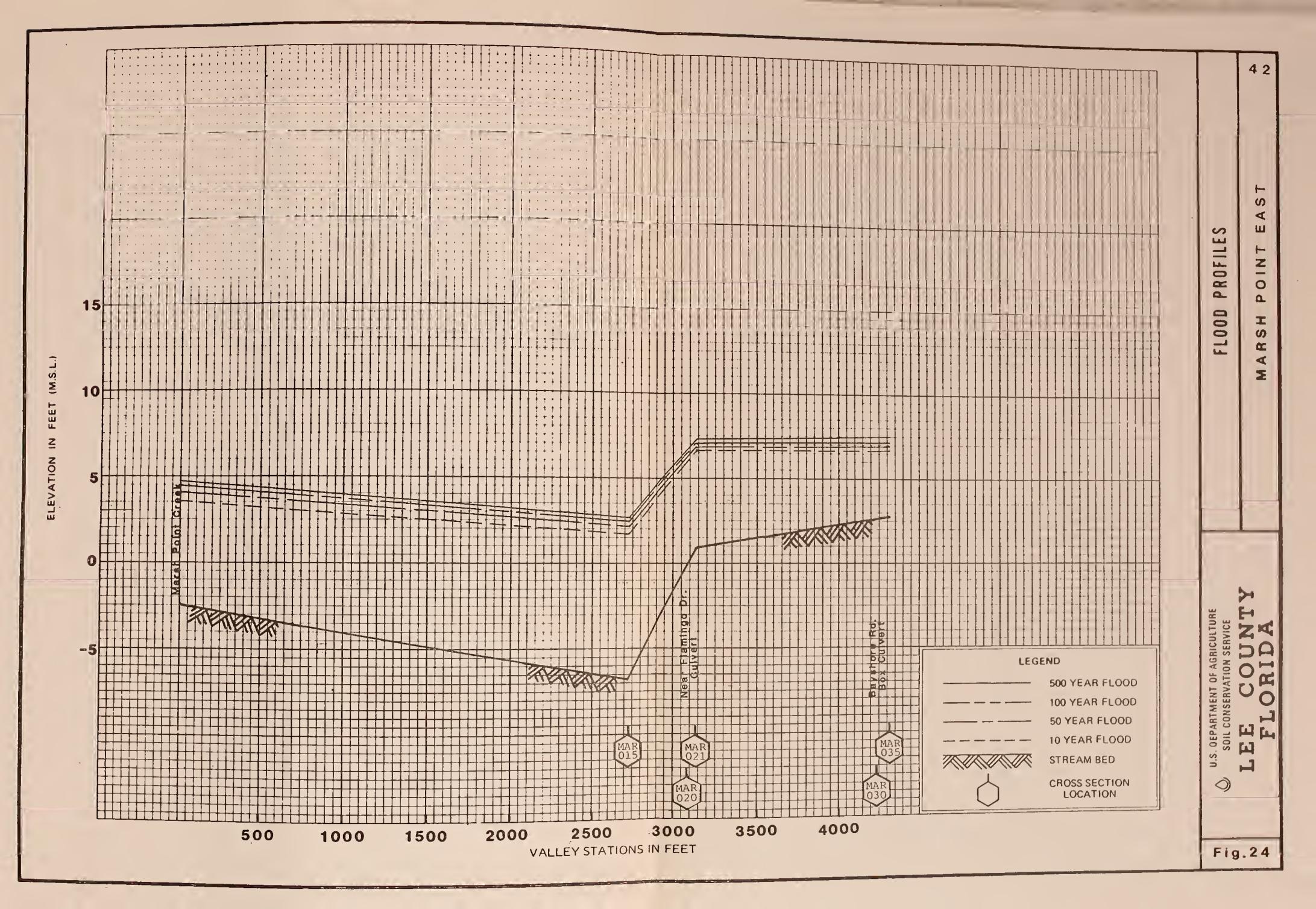




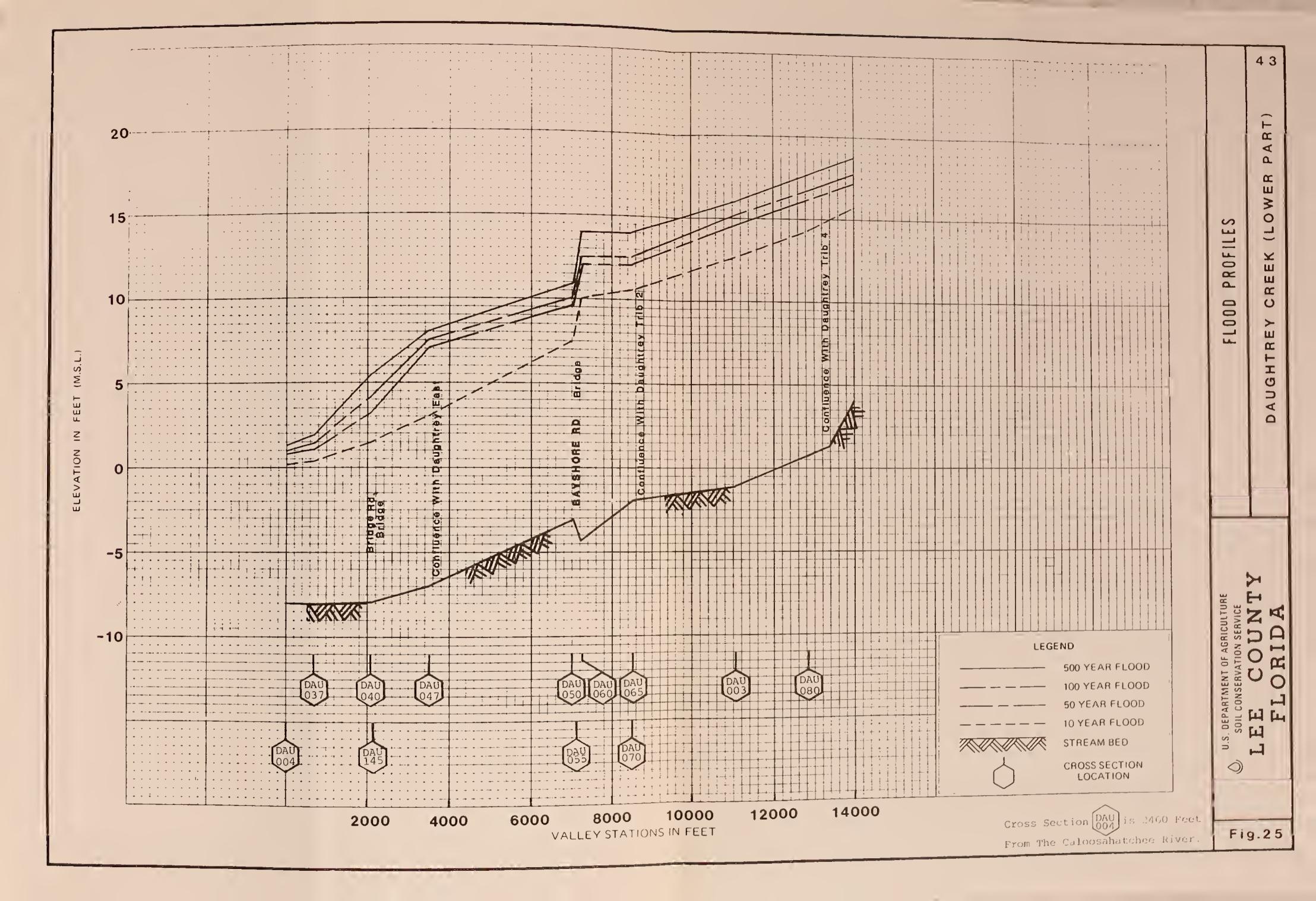




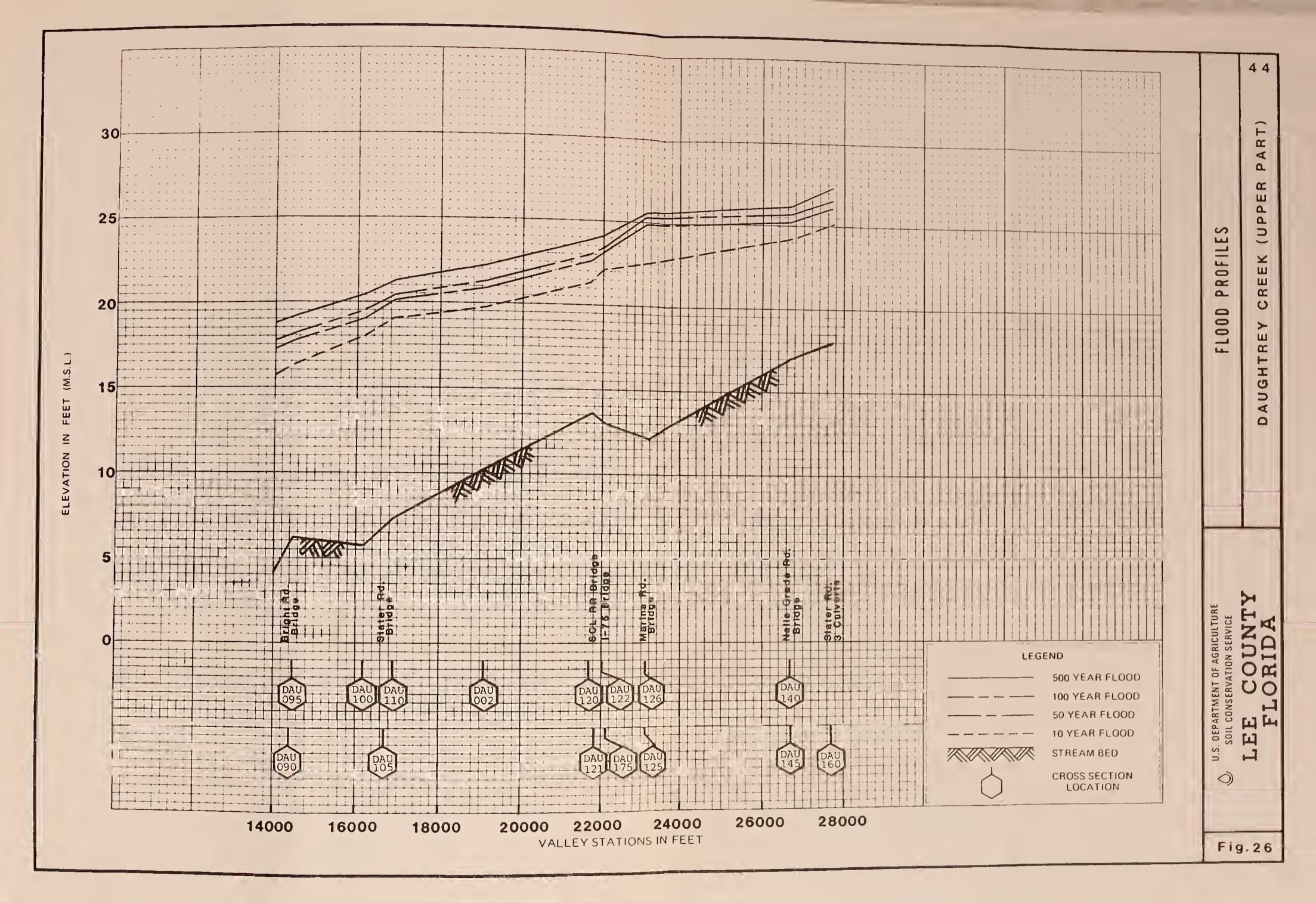




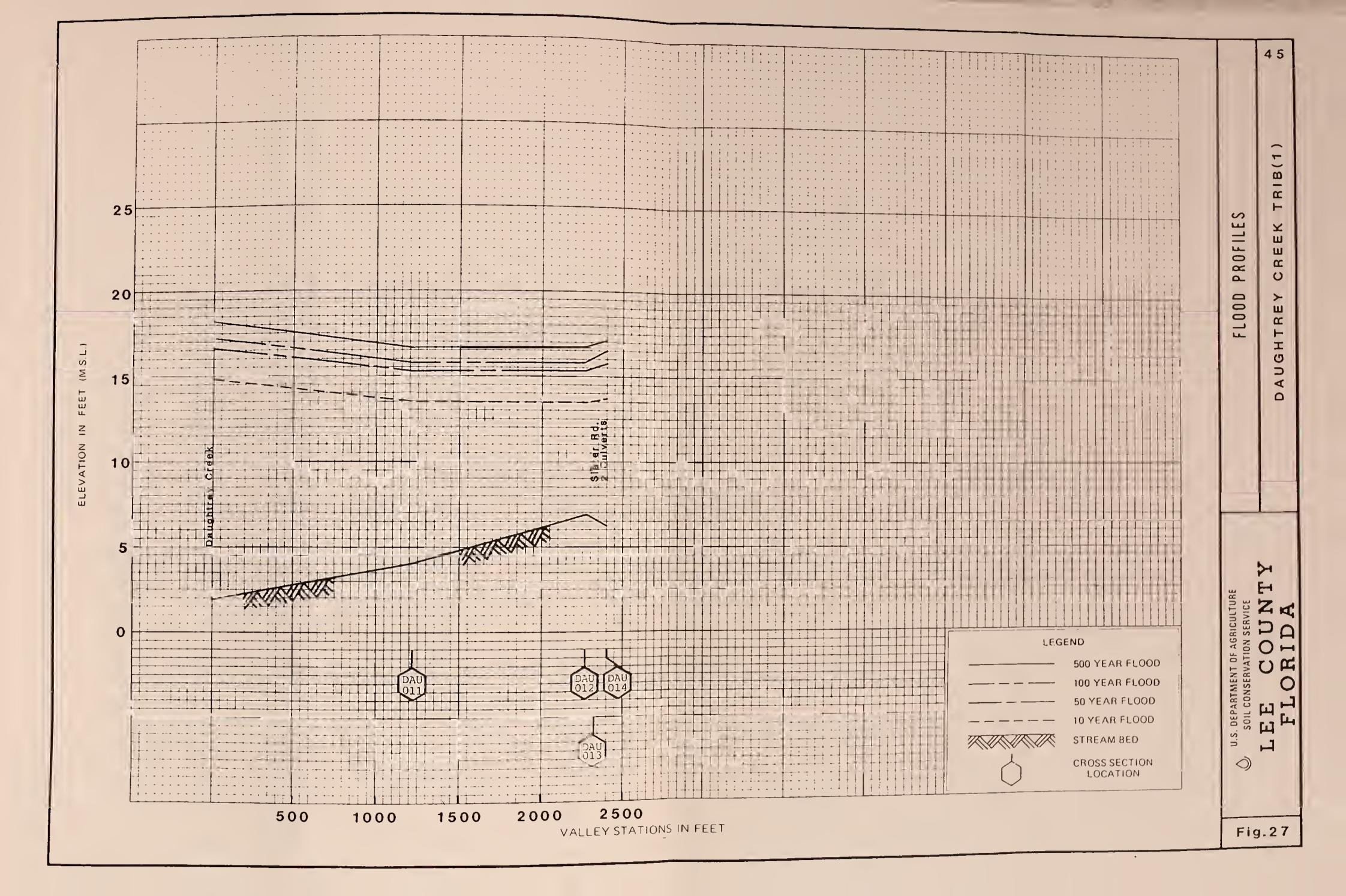




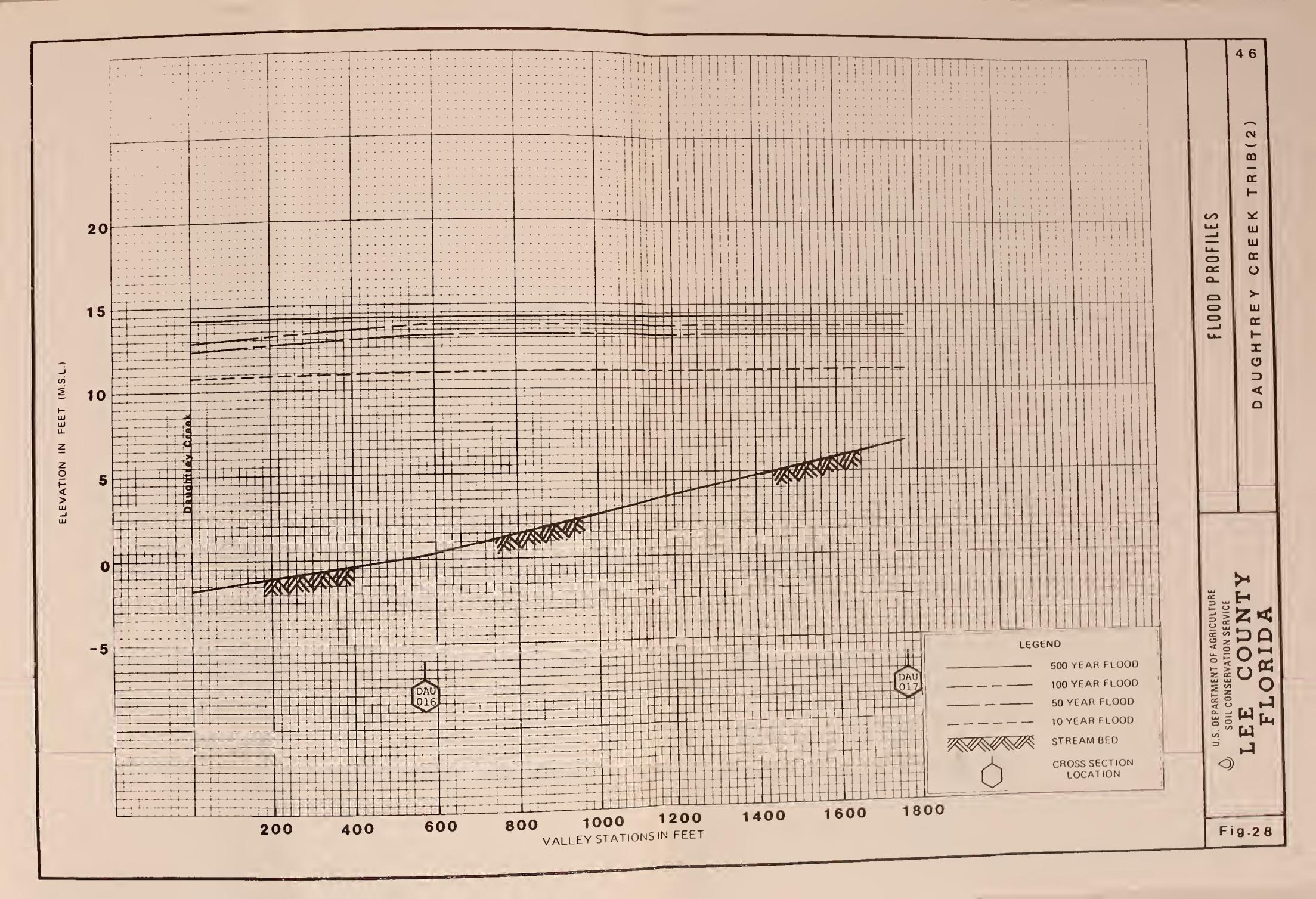




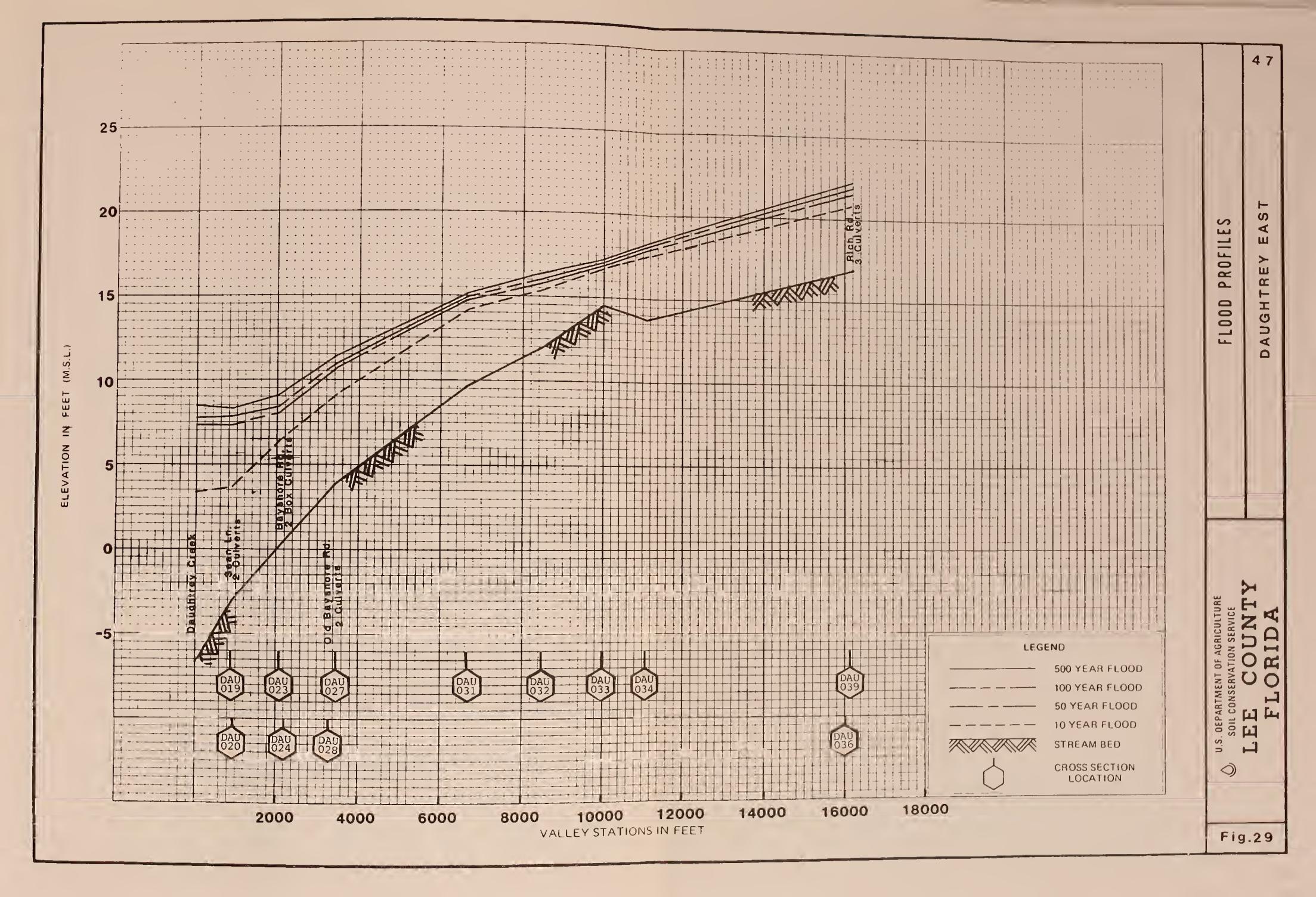




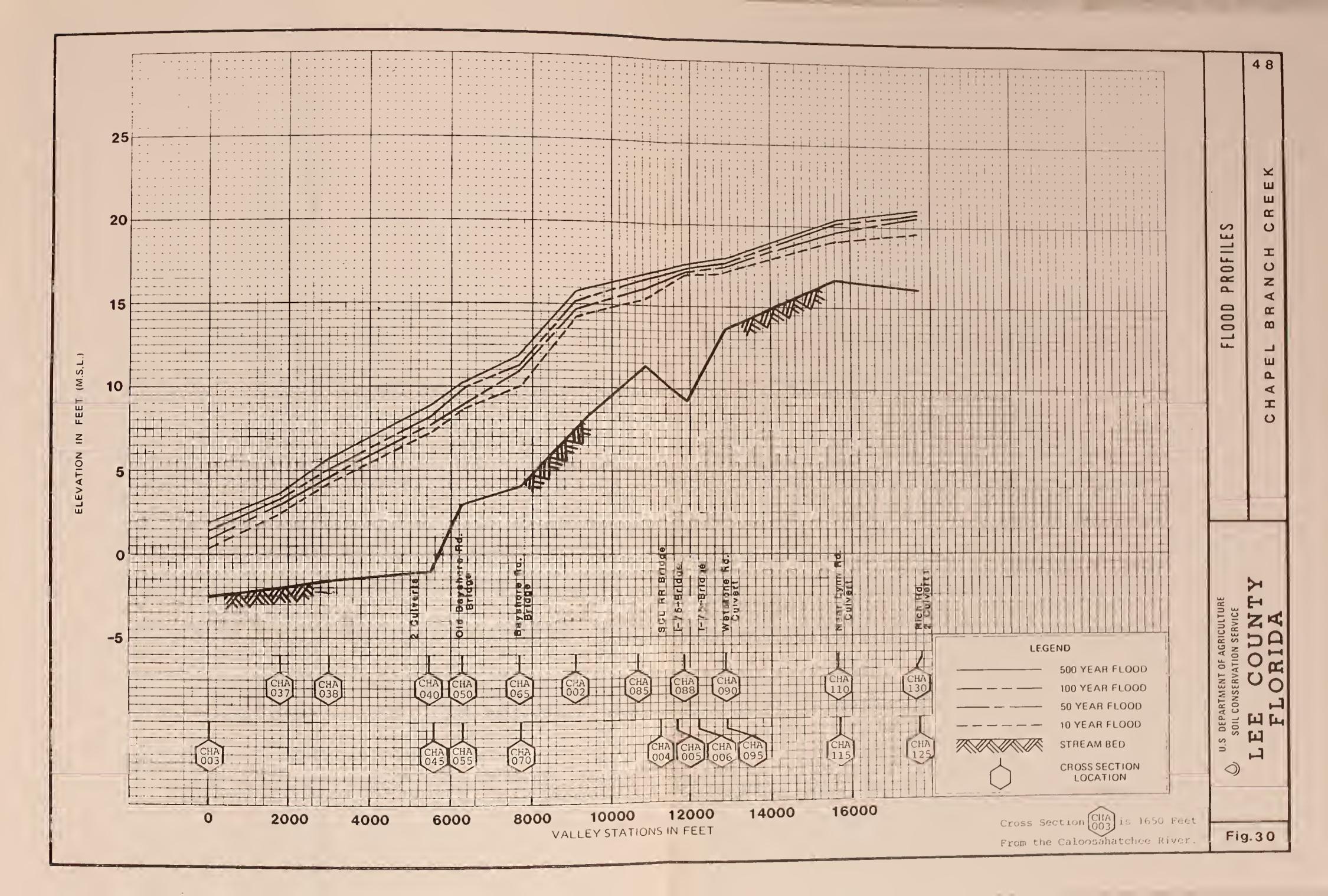




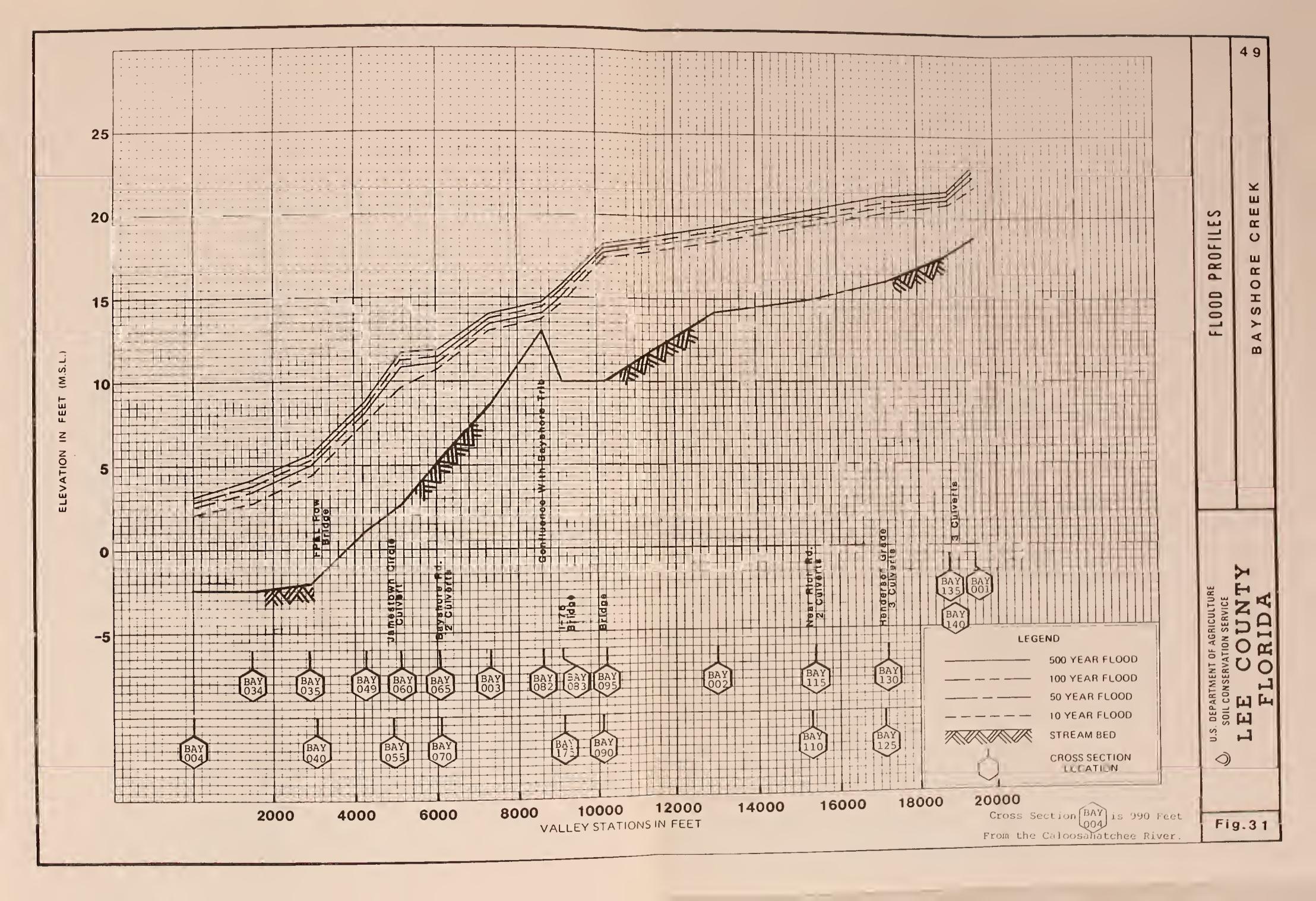




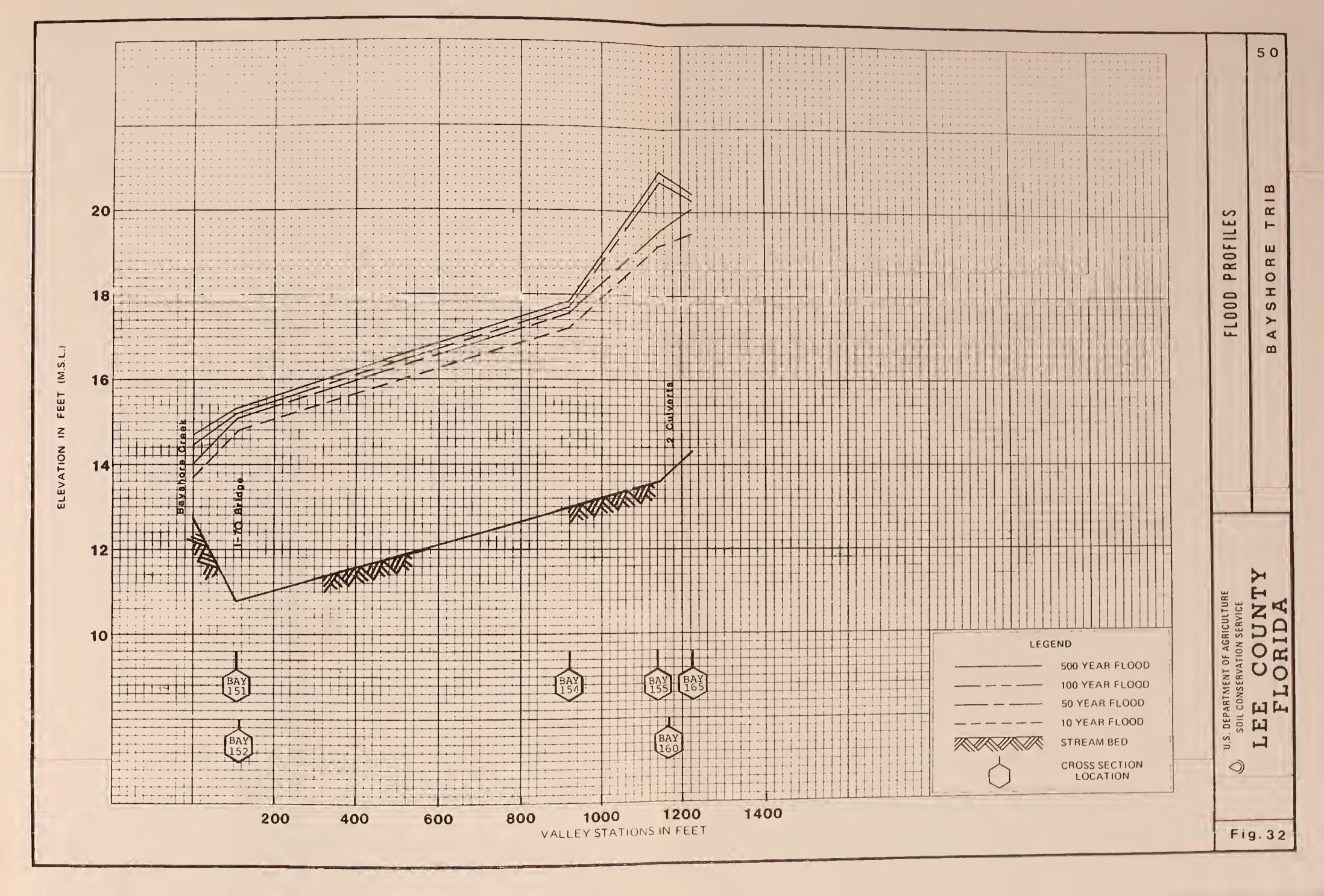




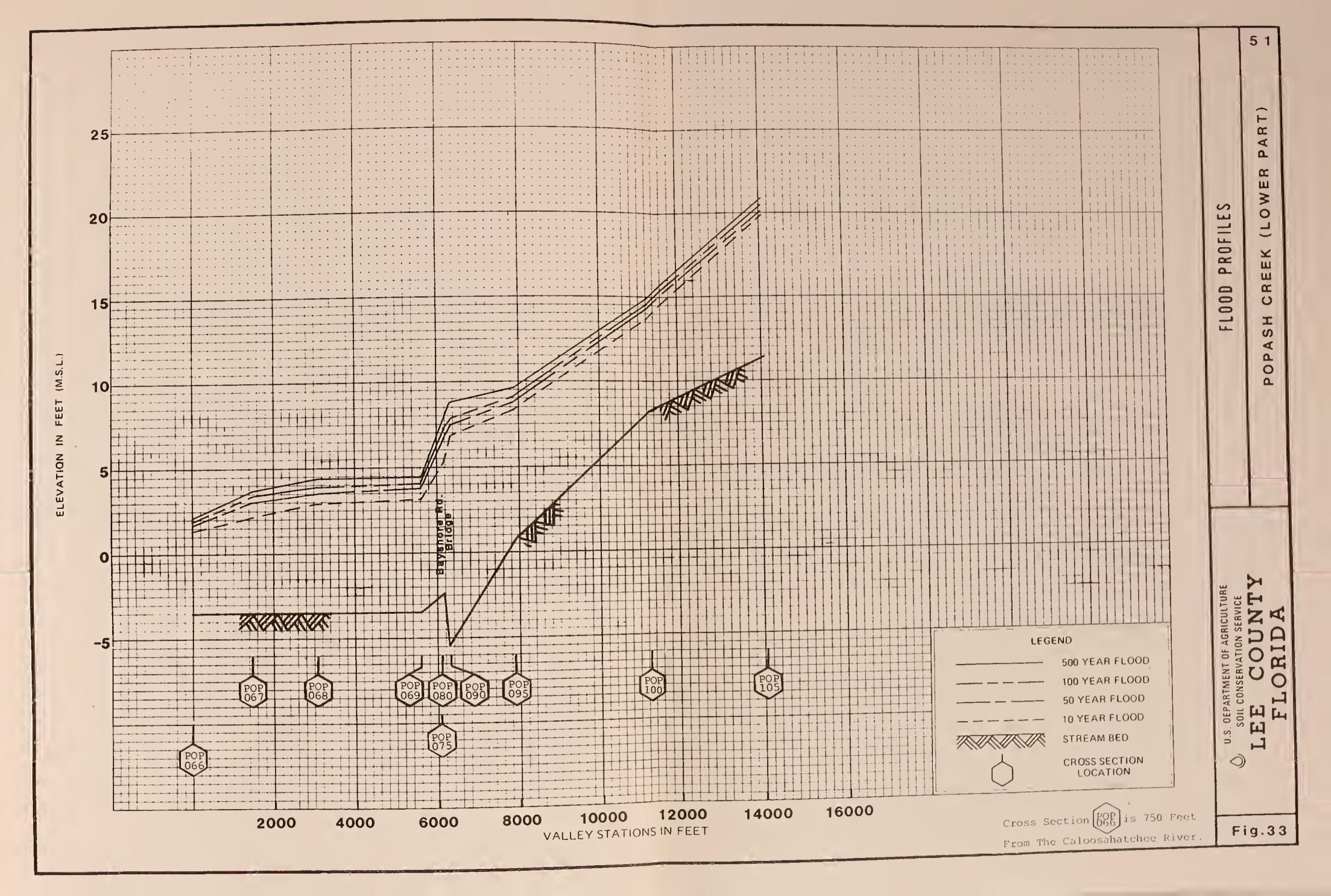




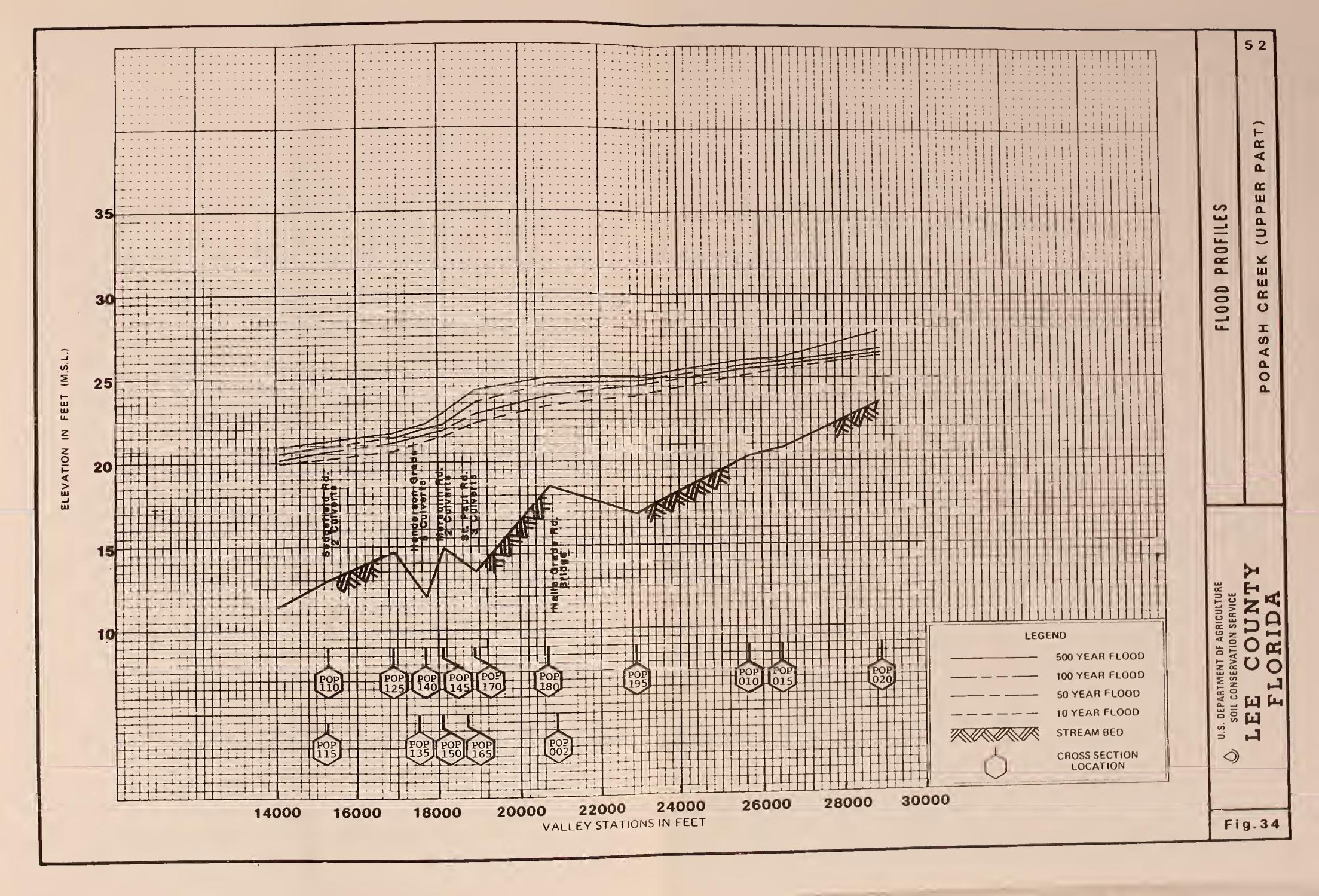




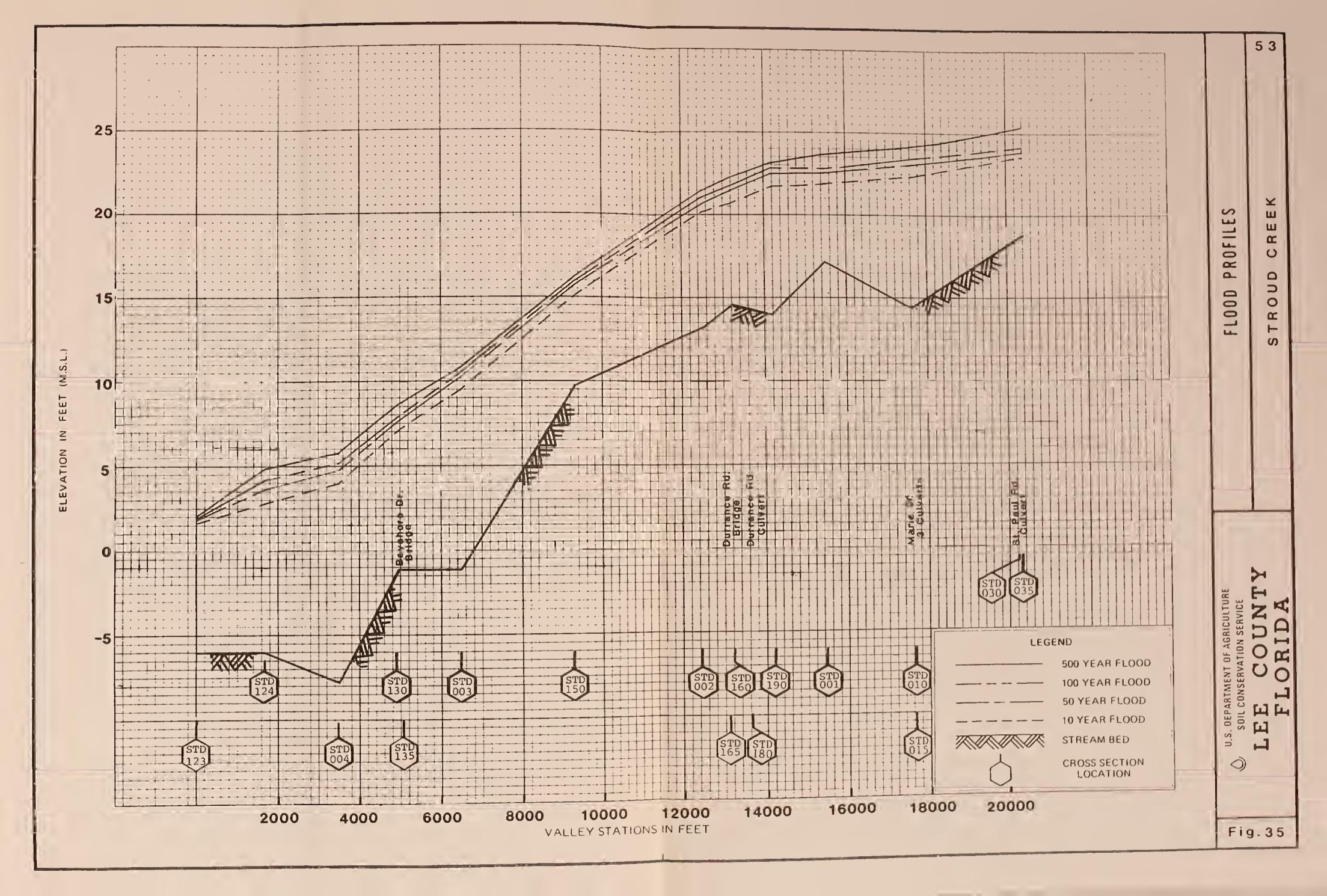




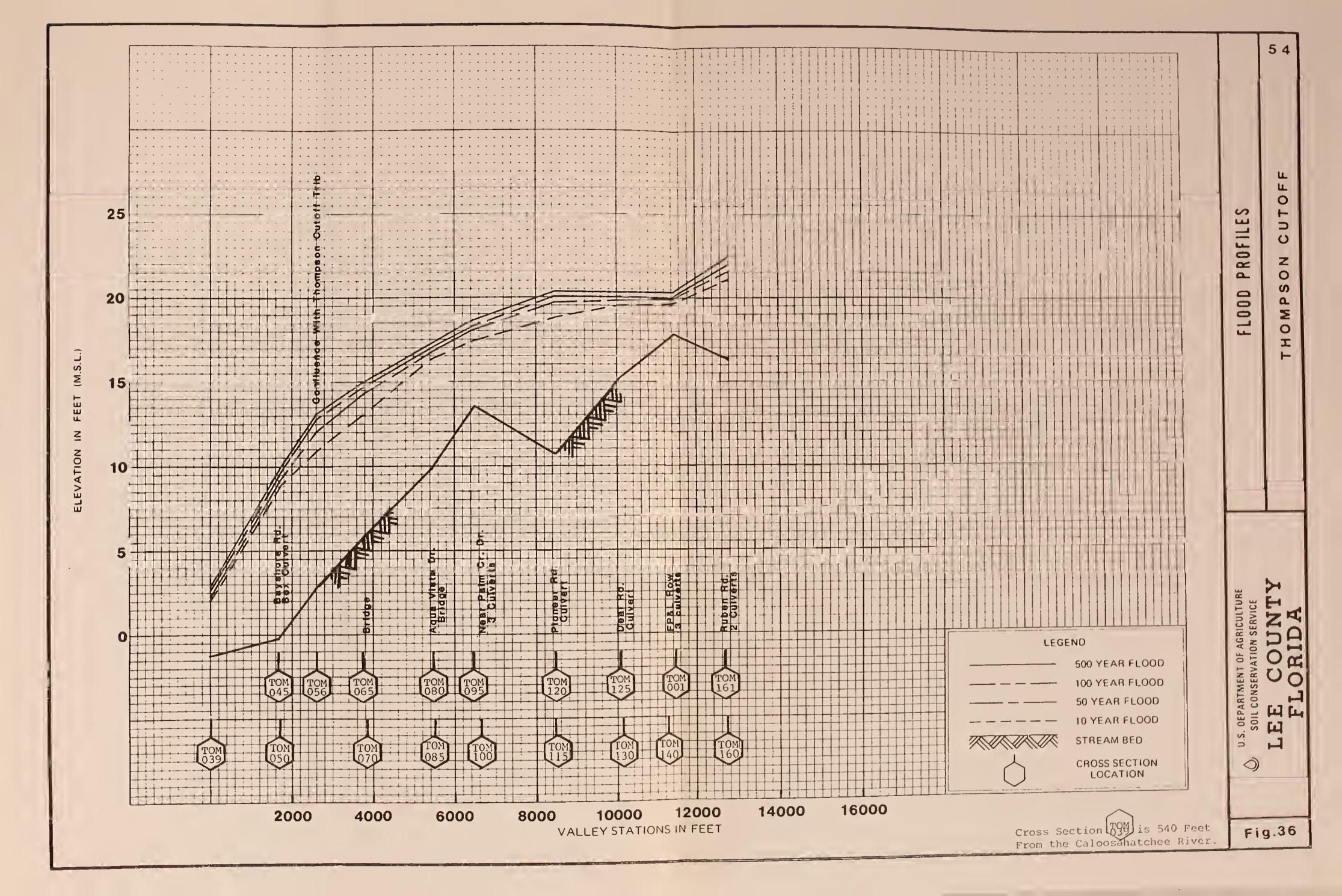




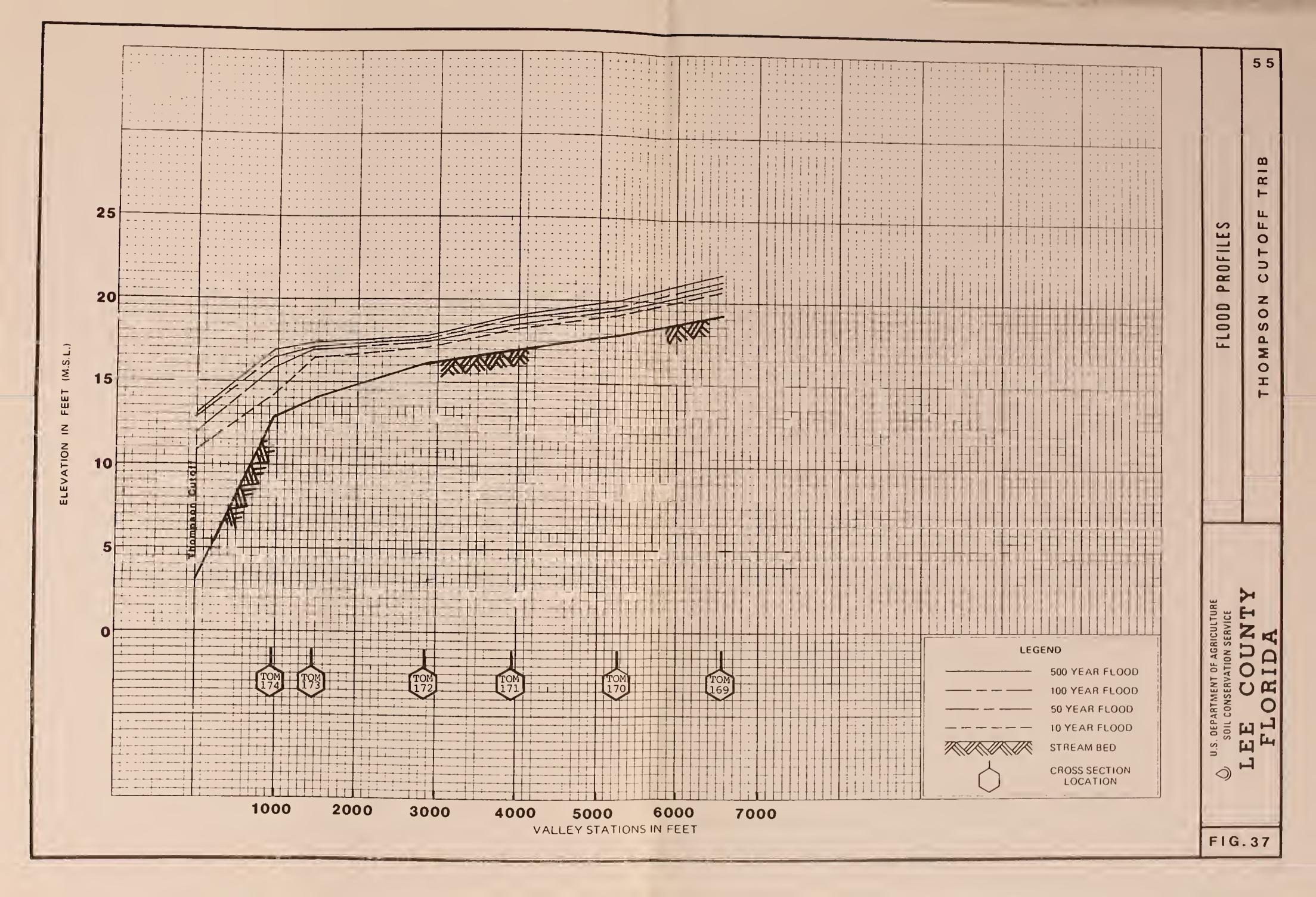








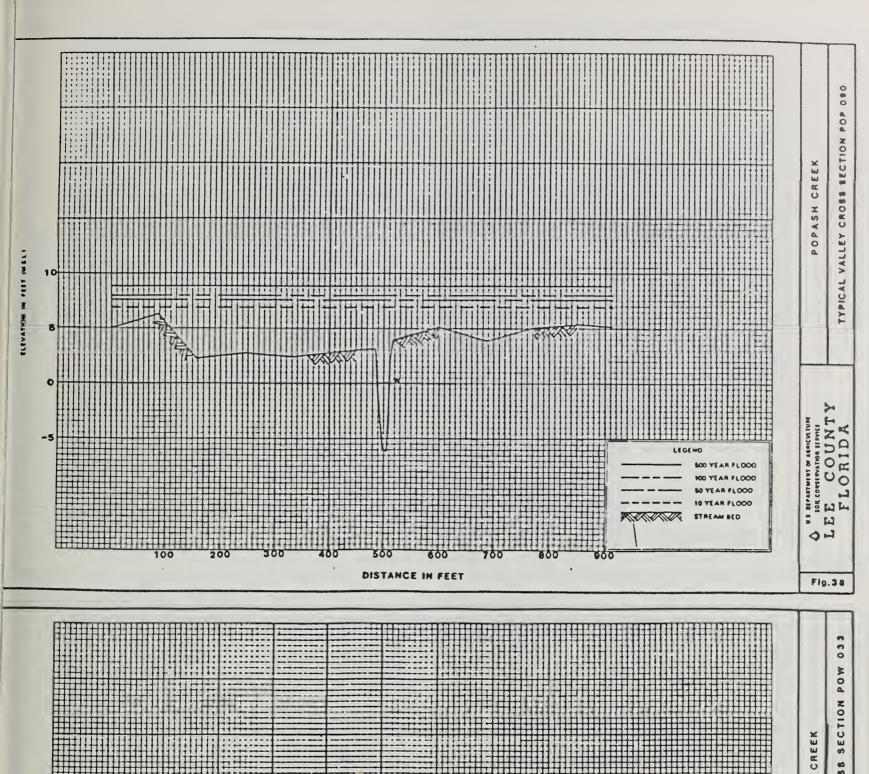


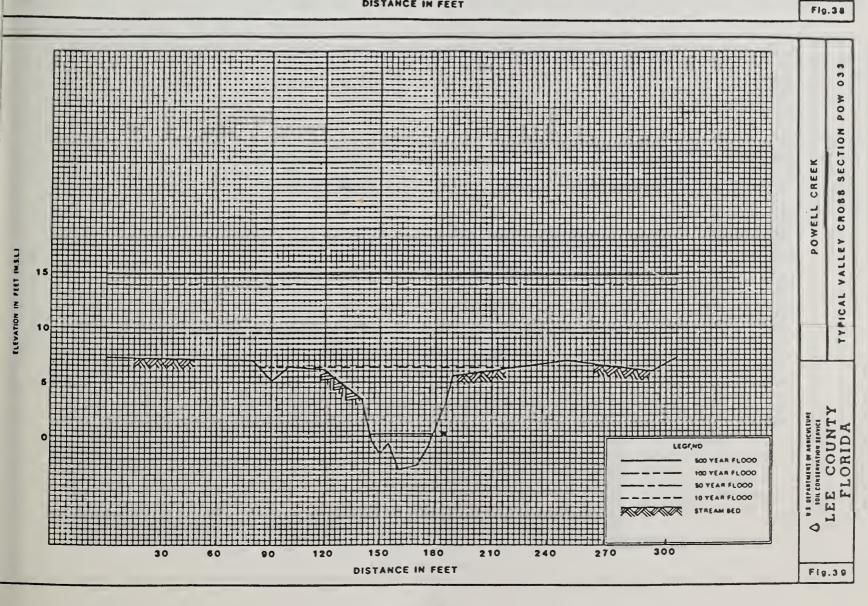




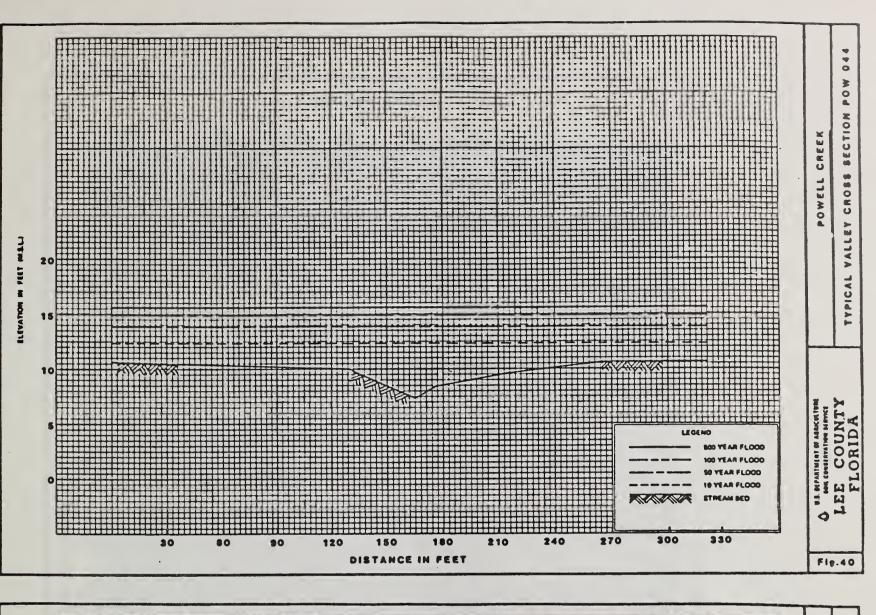
APPENDIX C

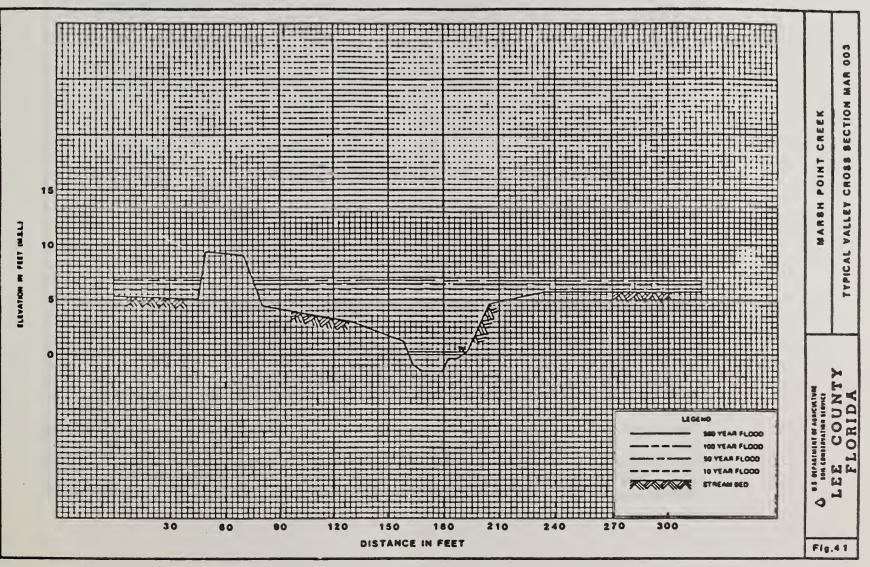
TYPICAL VALLEY CROSS SECTIONS

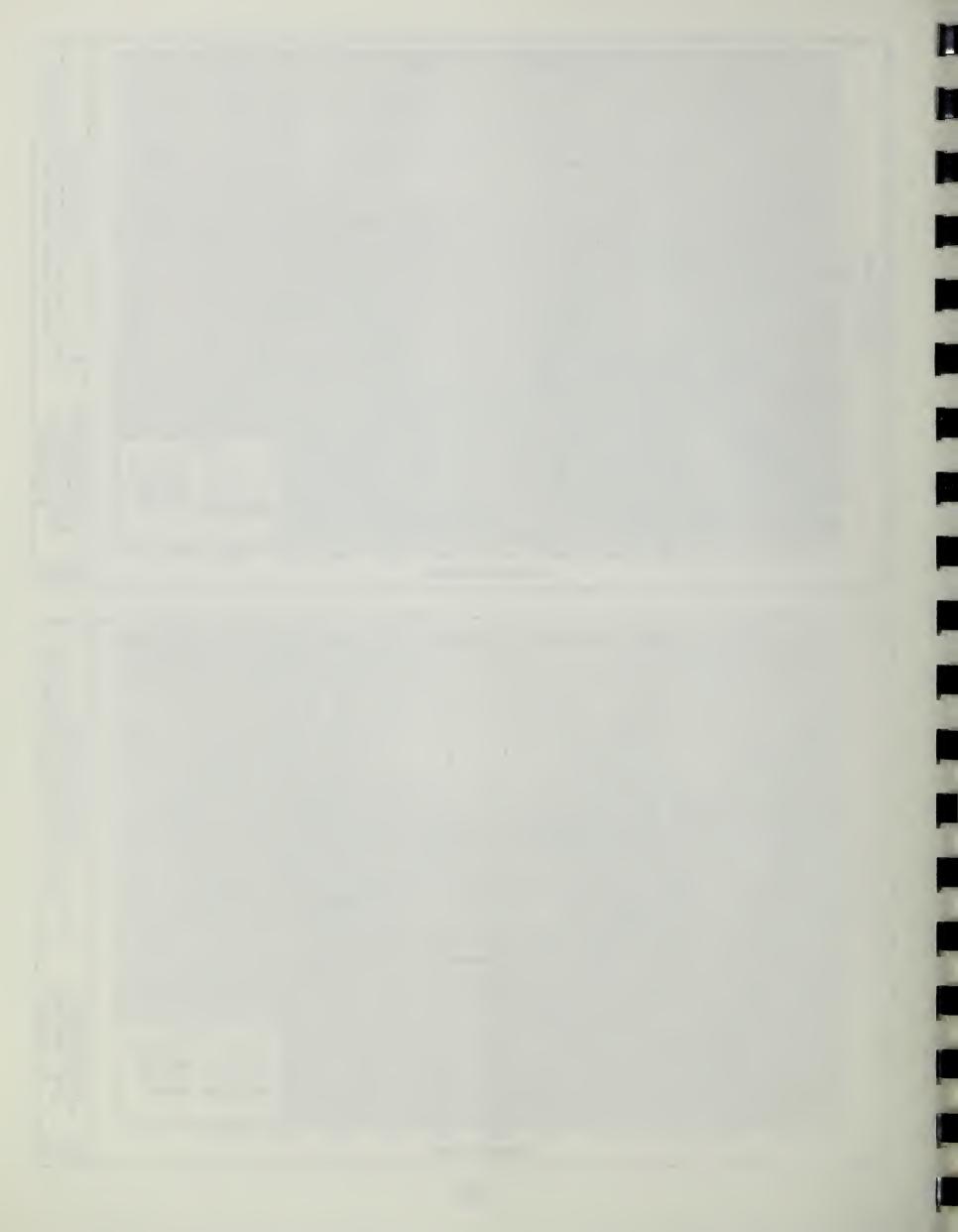


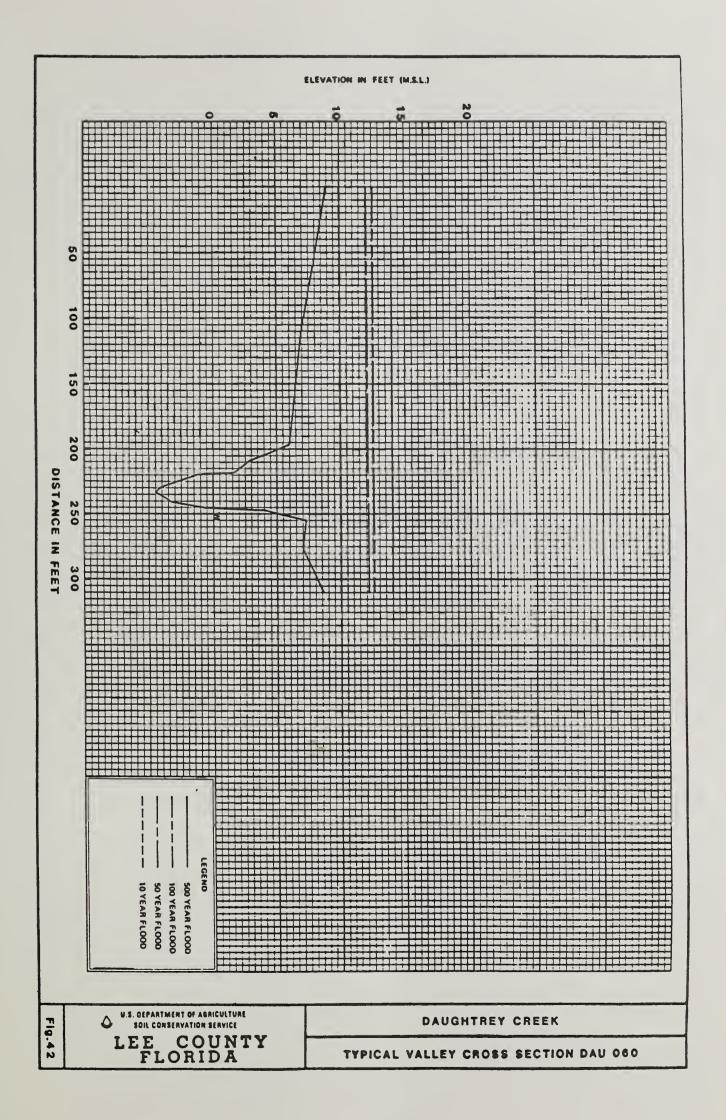














APPENDIX D

TECHNICAL APPENDIX



INVESTIGATIONS AND ANALYSES

This study was conducted in accordance with a plan of study dated March 19, 1980, by the SCS and local sponsors (see page 1). A review of pertinent literature was made by SCS personnel in order to become as familiar as possible with the complex hydrology of the study area. Bridge, culvert, and cross-section data were obtained by field survey by a private engineering firm under contract with the SCS and was field checked by SCS personnel. Supplemental base field data were obtained in the field by SCS and Lee County employees and estimated by photogrammetric methods. A topographic survey was prepared on a photo base with one foot contour intervals at a scale of 1 inch = 300 feet. This was done under a contract administered by the SCS and paid for by both Lee County and the SCS.

The SCS water surface profile program, WSP-2 (step backwater method), is used to determine water surface elevations for the range of discharge utilizing roughness coefficient data and the field data collected on cross sections, bridges, and culverts. Flood discharges are established by valley flood routings computed through use of the SCS computer program for Project Formulation Hydrology, TR-20. This program uses the modified ATT-KIN method for stream flow and valley flood routing.

On two of the creeks these programs yielded flood discharge rates that appeared excessive. After experimenting with other flood routing models, the USGS Water Resources Investigations 82-42 was chosen for analysis of Powell Creek and Water Resources Investigation 82-4012 was used for Daughtrey Creek.

The 100 year flood plain limits were delineated on the aerial photos with one-foot contour lines (see Appendix A). The width of the flood plain at each cross section was plotted with the area between cross sections interpolated.

Normal bridge flow conditions are assumed in making computations. No consideration is made for openings blocked by debris, flood plain filling or other encroachments which could affect the water surface profile. Computations for this study considered only those features in the flood plain at the time the field surveys were made. Additional watershed and flood plain development and/or stream modifications will require revised water surface profile computations. The methods used to determine the flood elevations are considered accurate within plus or minus 1/2 foot. Due to scale, however, some buildings on raised pads appear to be flooded when in actuality they will probably not.

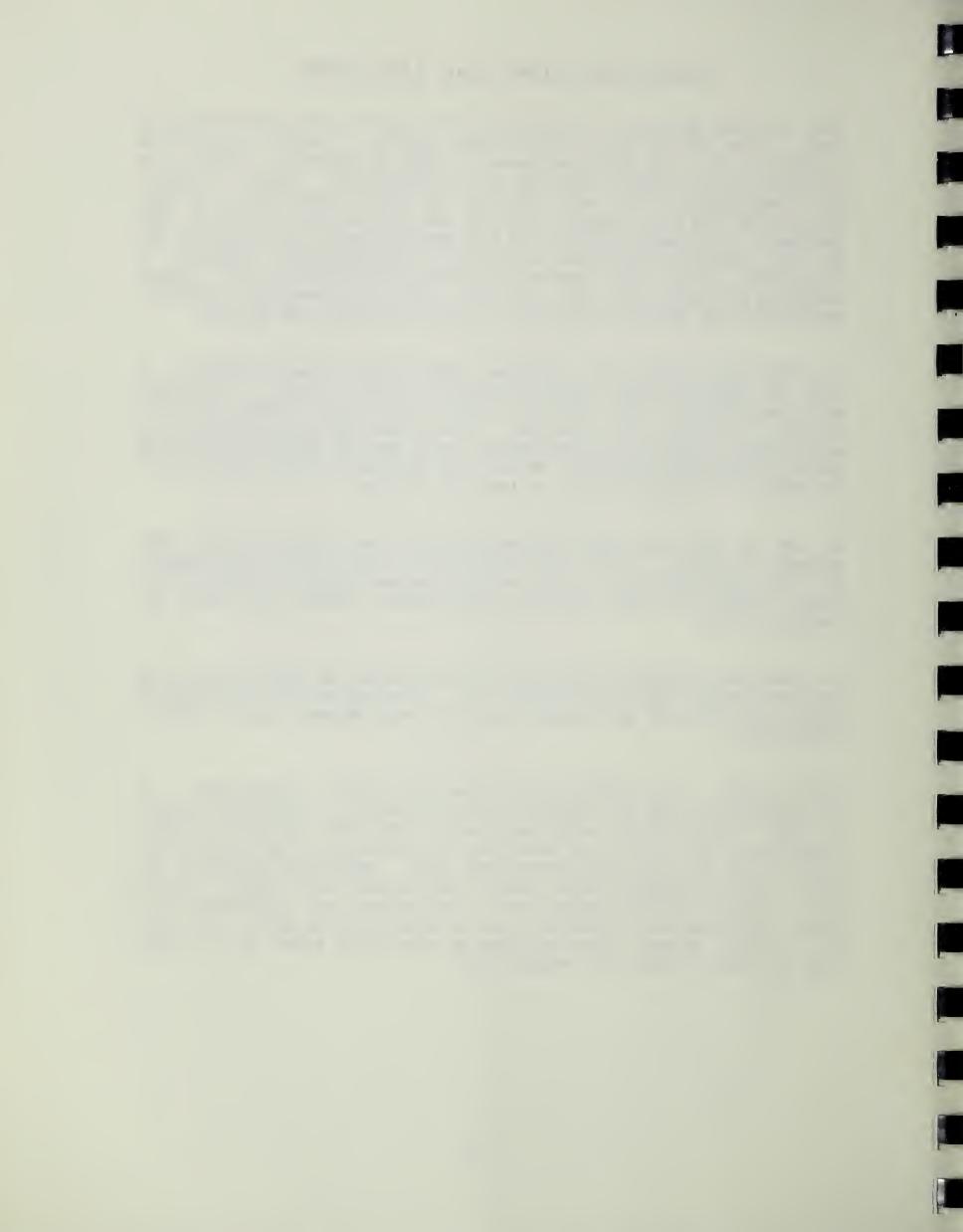


TABLE 3. DISCHARGE - ELEVATION - FREQUENCY DATA

L-2 (Trib to Yellow Found Control

ear	Discharge CFS	252	250	246	242	238	
500-Year	Elevation: Discharge MSL-Ft : CFS	9.3	11.7	12.2	13.1	13.6	
ear	Discharge CFS	200	198	195	192	188	
100-Year	Elevation: Discharge MSL-Ft. : CFS	8.9	11.2	.11.7	12.5	13.2	
	tion: Discharge:	179	177	174	172	168	
50-Year	Elevation: [MSL-Ft. :	8.7	10.8	11.3	12.0	12.6	
ear	Discharge CFS	122	121	119	117	115	
10-Year	Elevat MSL-F	8.2	9.6	10.1	10.8	11.4	
	Station Drainage &	0.36	0.34	0.31	0.28	0.25	
	Station	0	150	575	1037	1499	
	Cross Section	L-2090	L-2100	L-2105	L-2115	L-2125	

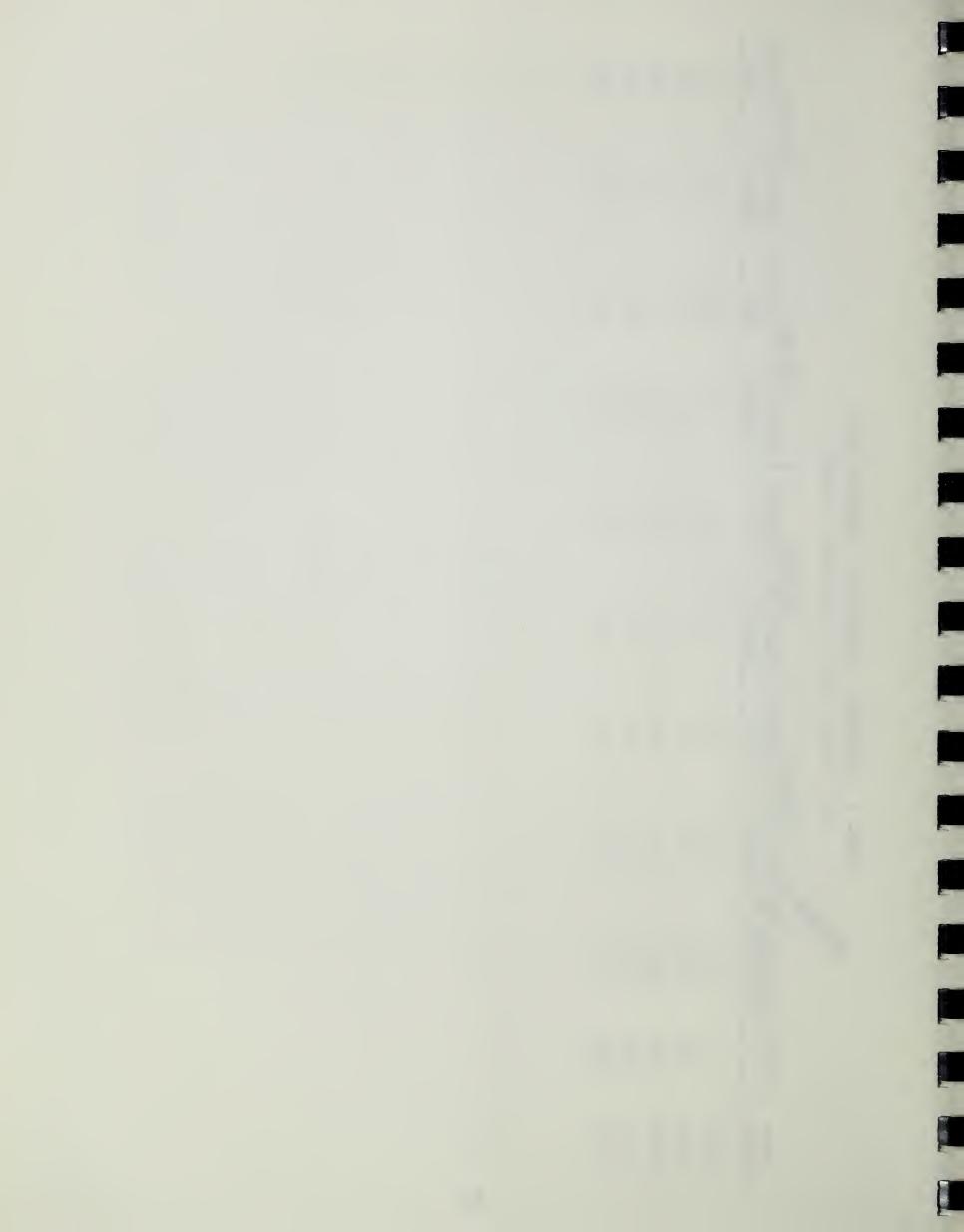


TABLE 4. DISCHARGE - ELEVATION - FREQUENCY DATA

Creek
Fever
Yellow
to
Trib
L-1

		7	10-Year	Year	: 50-Year	<u>.</u>	: 100-Year	lear	: 500-Year	Year
Cross		Statión Drainage Area(Mi ²)	MSL-F	Discharge CFS	Elevation: [MSL-Ft. :	Discharge CFS	Elevation: MSL-Ft.:	Discharge CFS	Elevat MSL-F	Discharge CFS
L-1005	0	0.84	3.0	550	3.9	776	4.2	856	4.7	1056
L-1015	999	0.73	5.1	531	5.8	756	6.1	835	6.5	1035
L-1020	778	0.70	8.3	531	9.6	756	6.6.	835	10.6	1035
L-1030	941	0.68	6.6	511	10.5	730	10.7	908	11.0	1001
L-1035	1541	0.68	10.0	511	10.7	730	ויוו	908	11.4	1001
L-1045	1641	0.66	10.1	491	10.8	703	11.3	778	11.6	967
L-1050	2241	0.64	10.9	472	11.5	229	11.8	750	12.1	934
L-1060	2341	0.62	11.7	453	12.4	651	12.7	722	13.0	901
G L-1065	2841	0.56	12.4	396	12.9	574	13.4	639	13.6	805
6 L-1075	2941	0.54	12.5	377	13.0	549	13.5	612	13.7	769
L-1080	3741	0.51	13.6	350	14.2	512	14.6	571	14.9	720
L-1086	3841	0.49	14.5	350	15.1	511	15.6	571	15.9	720

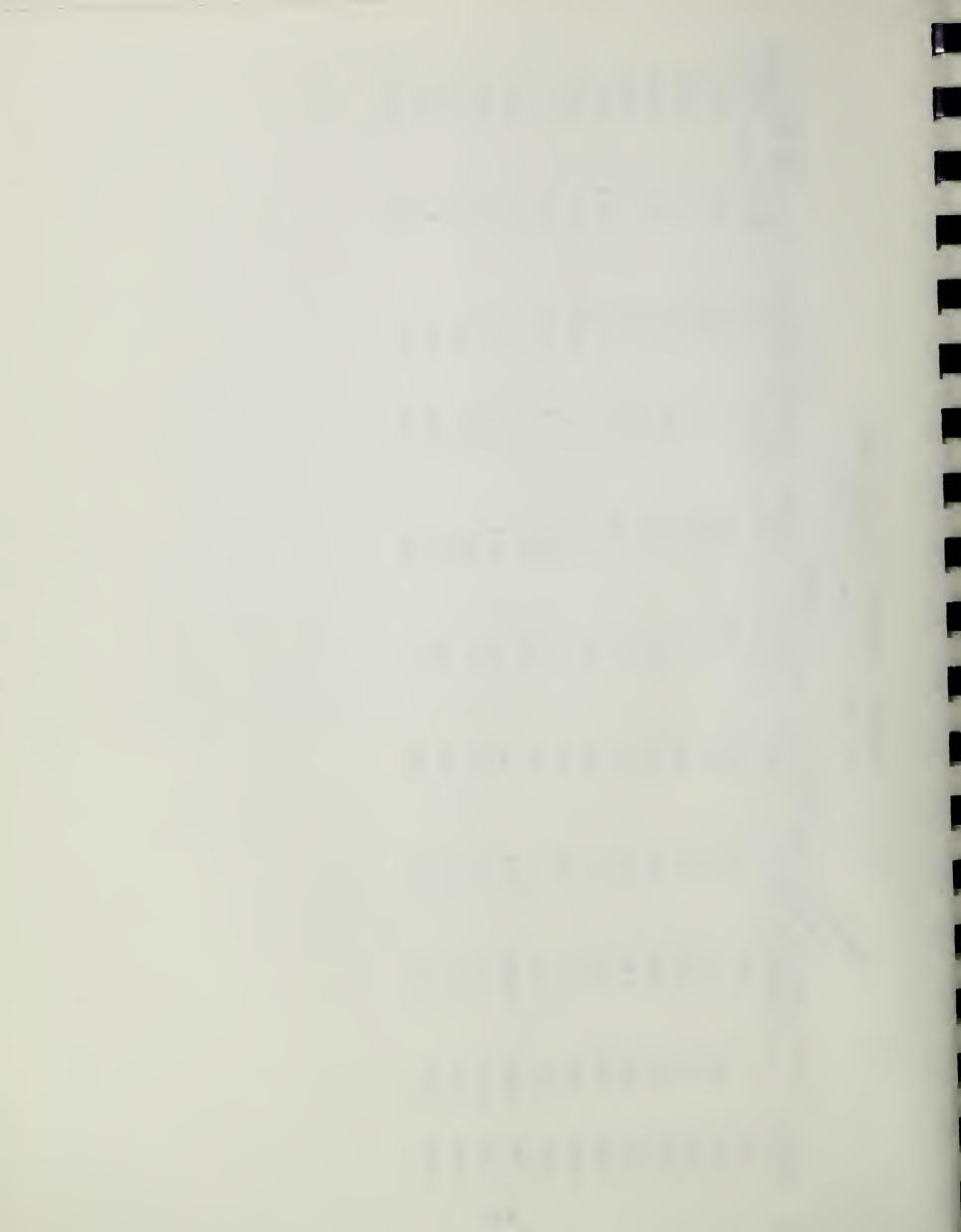


TABLE 5. DISCHARGE - ELEVATION - FREQUENCY DATA Powell Creek (with Suncoast Canal)

		•	3000	10-Year	50-Year		100-Year		3 COV. OOB	S
Cross	Station	Drainage Area(Mi	Elevati MSL-Ft	n: Discharge CFS	Elevation: MSL-Ft.	Discharge CFS	Elevation: MSL-Ft.:	Discharge CFS	Elevation: MSL-Ft :	Discharge CFS
P0W030	0	13.03	1.1	1665	1.3	3828	1.4	5247	1.5	7871
P0W032	100	12.86	2.3	1643	2.5	3771	2.8	5168	3.1	7752
P0W033	2650	9.64	6.4	1217	9.5	2715	13.9	3699	14.9	5549
P0W035	2750	9.62	6.7	1215	10.1	2708	14.0	3690	15.0	5535
P0W036	4025	9.56	8.8	1207	11.3	2689	14.1	3664	15.1	5496
P0W038	4125	9.54	10.2	1204	11.8	2683	14.2	3655	15.1	5483
P0W039	6300	9.10	11.8	1146	13.4	2542	14.7	3460	15.2	5190
P0W040	6350	60.6	11.9	1145	13.5	2539	14.7	3456	15.3	5184
POW041	6400	9.04	12.1	1138	13.7	2523	14.8	3433	15.5	5150
P0W042	7300	9.03	12.3	1137	13.8	2520	14.9	3429	15.6	5144
P0W043	, 7600	1.04	12.3	120	13.8	214	14.9	279	15.6	419
P0W044	8425	1.03	12.3	118	13.8	212	14.9	276	15.6	419
P0W045	10045	0.93	12.3	106	13.8	188	14.9	245	15.6	368
P0W047	10215	0.91	12.5	104	13.9	184	15.0	239	15.6	359
P0W048	14040	0.75	13.4	85	14.4	147	15.1	191	15.7	287
P0M050	14140	0.73	13.4	83	14.9	143	15.7	185	16.2	278
P0W051	16690	0.61	13.5	89	15.5	116	15.9	150	16.5	225
POW053	16790	0.59	14.2	99	15.9	112	16.1	144	16.6	216
P0W054	17990	0.48	15.4	53	16.2	88	16.3	113	16.7	170
P0M055	20540	0.12	15.4	12	16.2	18	16.4	22	16.8	33

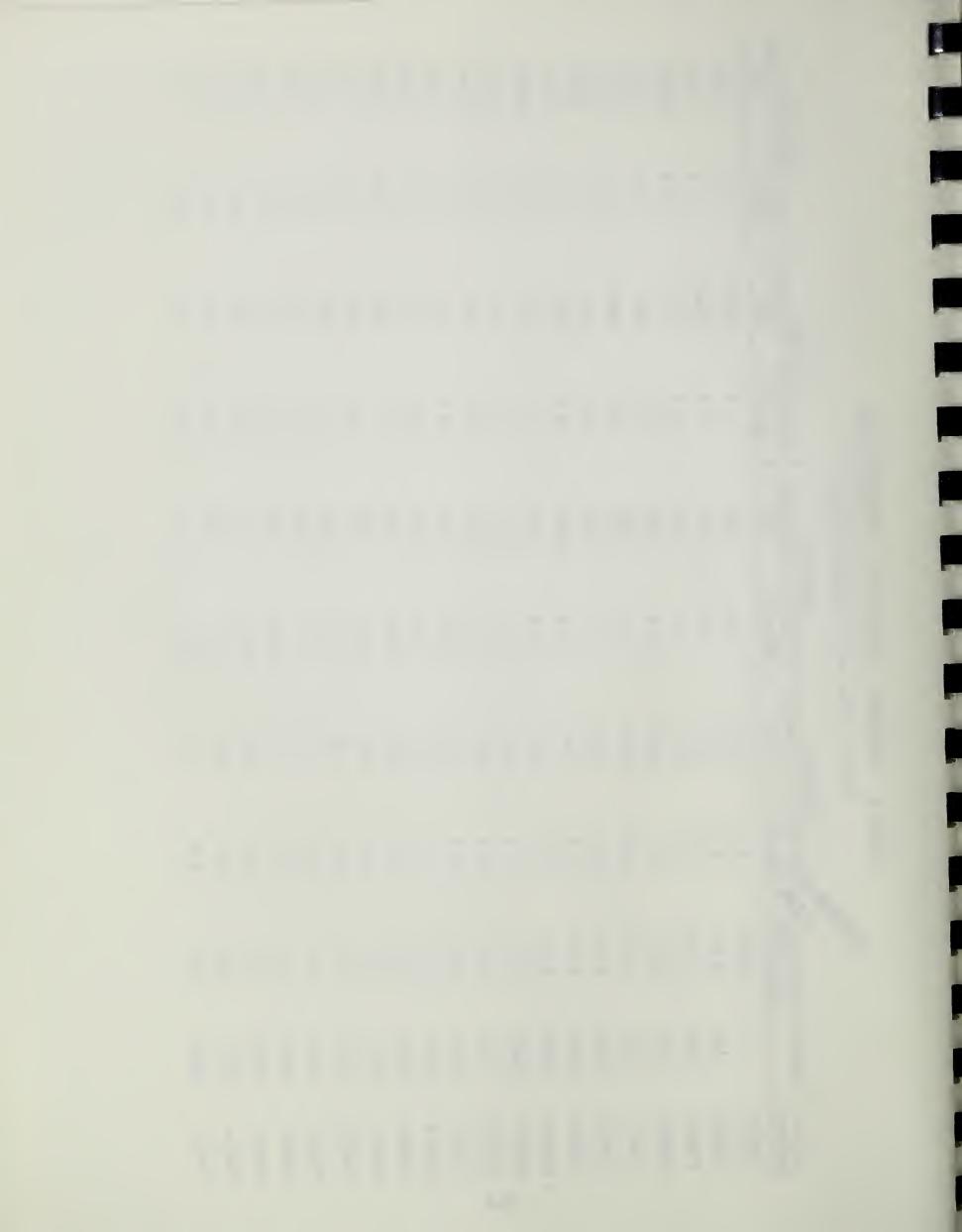


TABLE 6. DISCHARGE - ELEVATION - FREQUENCY DATA Powell Creek Trib (Old Railroad Grade)

•		9 10 0 0	1-01	10-Year	50-Year	ar :	100-Year	ear	500-Year	ear
Cross Section	Station	Station Drainage (Mi ²)	2) MSL-Ft. : CFS	Discharge CFS	! Elevation: Discharge ! MSL-Ft. : CFS	Jischarge CFS	Elevation: Discha MSL-Ft.: CFS	Discharge CFS	Elevation: Discharge MSL-Ft : CFS	Discharge CFS
P0W020	1170	7.85	14.1	983	15.6	2148	16.4	2915	17.1	4373
POW021	2520	7.72	14.8	996	15.9	2107	16.6	2859	17.2	4289
P0W022	5220	7.56	16.7	945	17.5	2058	18.1	2790	18.5	4185
P0W023	7920	7.46	16.9	932	17.9	2027	18.4	2748	18.9	4122
P0W024	12195	6.46	18.0	803	18.5	1720	18.9	2325	19.4	3488
P0W025	16320	3.80	18.9	462	19.2	939	19.5	1256	24.7	1884

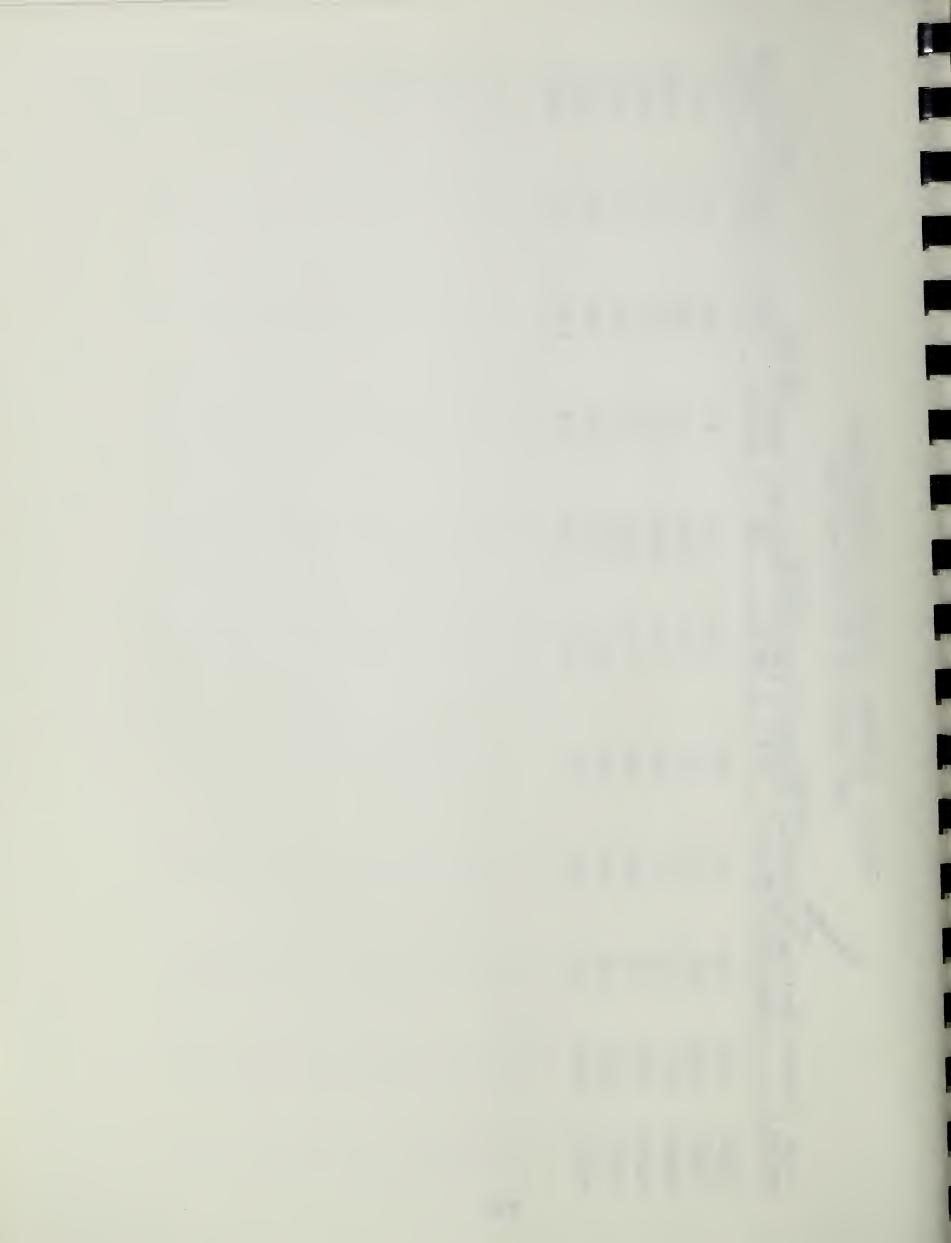


TABLE 7. DISCHARGE - ELEVATION - FREQUENCY DATA

Creek Trib (US 41 Canal)	
Powell Cr	
400	

		/ e /	5/0/	10-Year	; 50-Year	ar :	100-Year	ear	; 500-Year	ear
Cross Section	Station	Cross Statión Drainage Section : Area (Mi ²)	* Elevat MSL-F	Discharge CFS	Elevat MSL-F	Discharge CFS	Elevation: Discharge MSL-Ft.: CFS	Discharge CFS	Elevation: Discharge MSL-Ft : CFS	Discharge CFS
POWOOJ	2400	3.12	10.9	376	12.1	750	12.2	666	15.2	1499
P0W002	5250	3.05	13.7	367	14.7	731	15.2	973	15.9	1460
P0W003	7470	2.86	15.2	344	16.2	629	16.3	903	16.7	1355
P0W004	11070	2.78	16.0	334	16.9	657	17.1	874	17.5	1311
P0W005	13530	2.31	16.9	275	18.1	532	18.3	705	18.7	1058
P0W006	17730	2.13	17.6	, 253	18.3	485	18.6	642	18.9	963
P0M007	20580	1.59	18.4	186	18.7	347	19.0	457	19:4	989
P0W008	23760	1.40	18.9	163	19.3	300	19.3	394	19.8	591

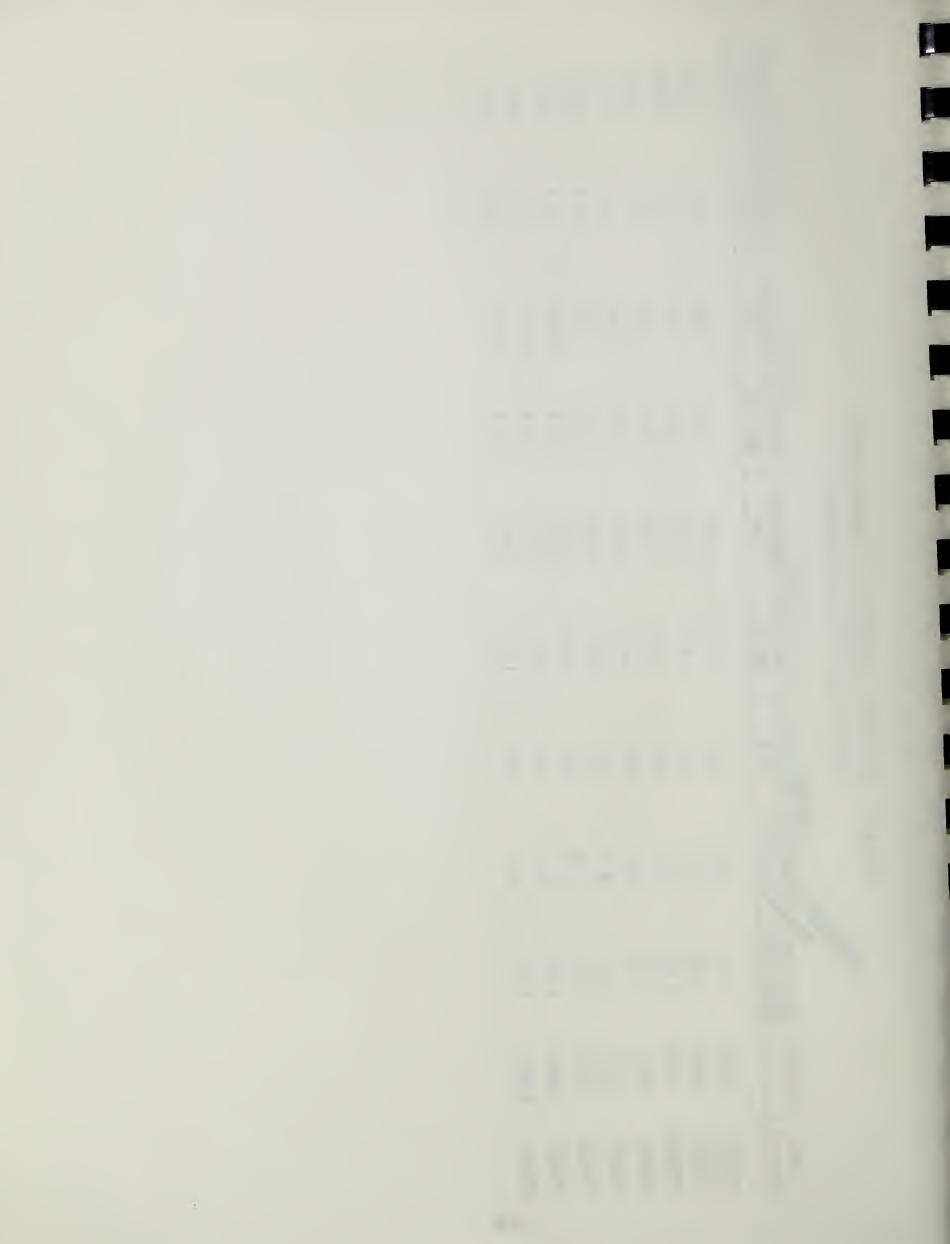


TABLE 8. DISCHARGE - ELEVATION - FREQUENCY DATA Marsh Point Creek

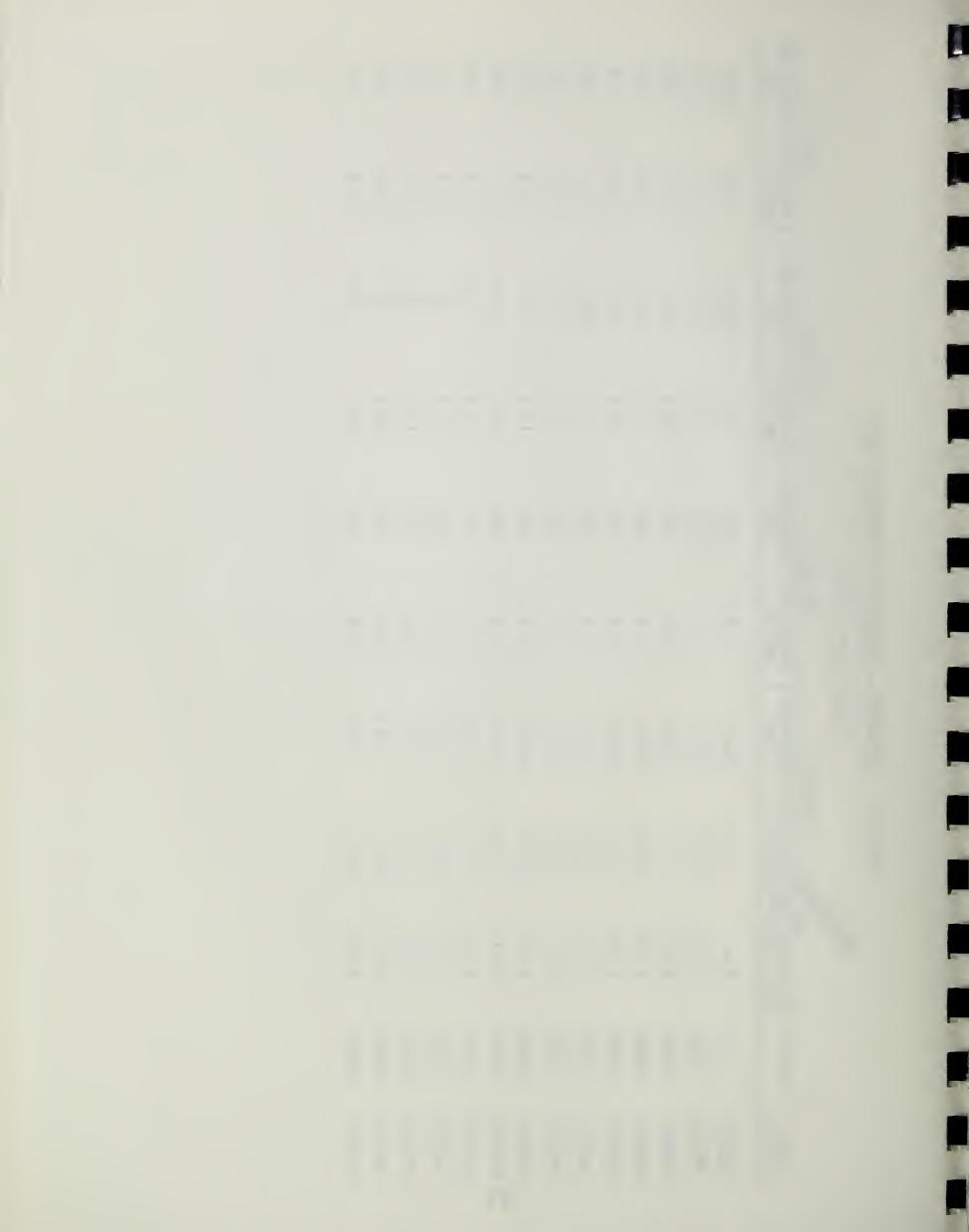


TABLE 9. DISCHARGE - ELEVATION - FREQUENCY DATA

	Discharge CFS	2751	200
500-Year		27	۵,
50	levation MSL-Ft	5.4	6.9
	ge		
ear	Discharge CFS	2196	399
100-Year	Elevation: MSL-Ft.	5.3	6.7
ear	Discharge CFS	1973	358
50-Year	<pre>Elevation: MSL-Ft. :</pre>	5.1	6.4
10-Year	Elevation: Discharge MSL-Ft. : CFS	1366	247
10-	Elevation: MSL-Ft. :	4.9	0.9
• •	Drainage Area (Mi ²)	0.11	0.10
	Station Drainage Area (Mi	22500	22600
	Cross Section	MAR010	MARO11

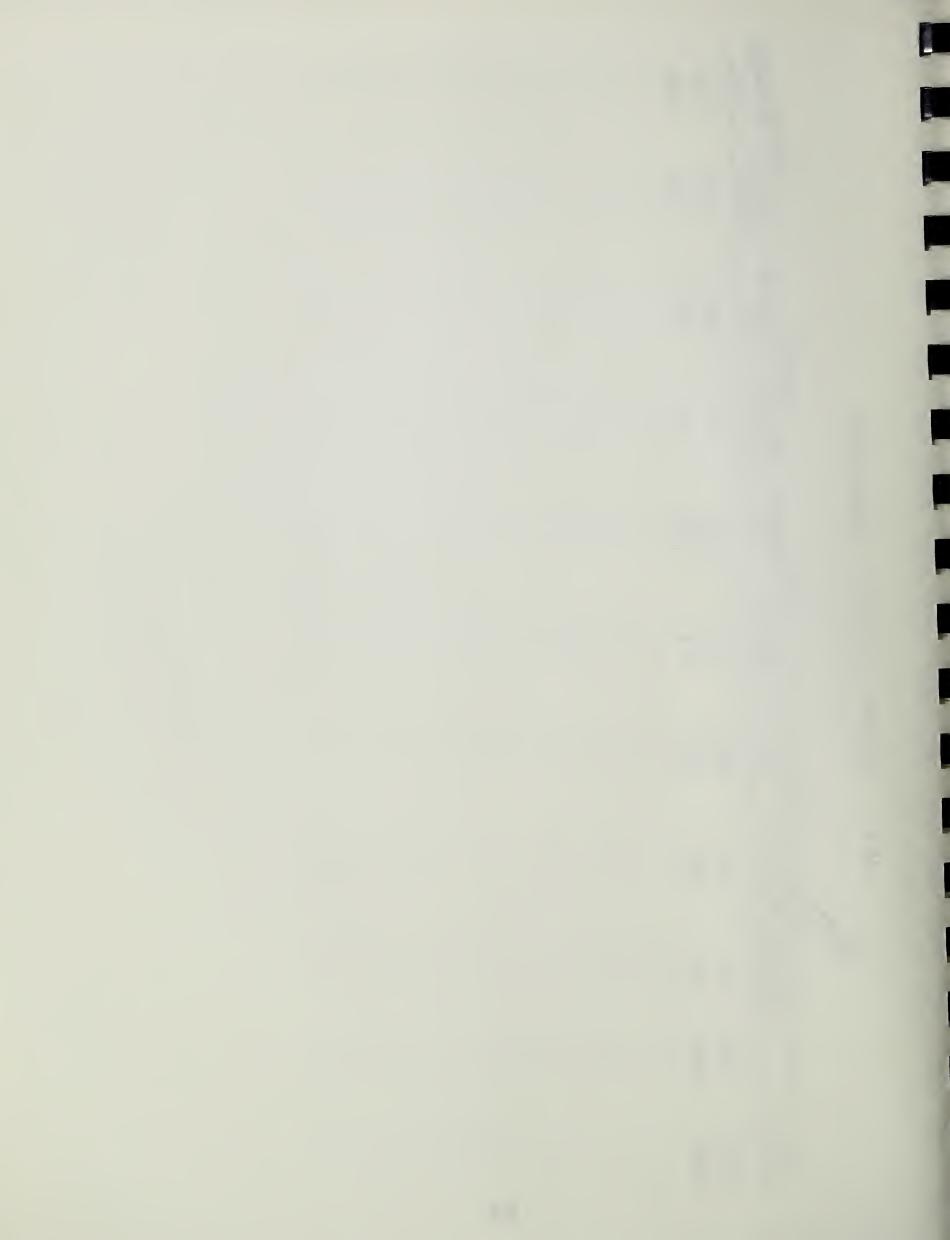


TABLE 10. DISCHARGE - ELEVATION - FREQUENCY DATA Marsh Point East

Year	Elevation: Discharge MSL-Ft : CFS	819	119
500-Year	Elevation: MSL-Ft :	7.1	7.2
100-Year	levation: Discharge MSL-Ft.: CFS	647	92
100-	<pre>Elevation: MSL-Ft. :</pre>	7.0	7.1
ar	ation: Discharge -Ft.: CFS	278	85
50-Year	<pre>Elevation: MSL-Ft. :</pre>	6.9	7.0
10-Year	Discharge CFS	391	28
10-1	Flevation:	6.7	6.8
9 \$2 9	Orainage (Ai2)	0.49	0.46
	Station Drainage Area (Mi	2835	3885
•	Cross Section :	MAR021	MAR025

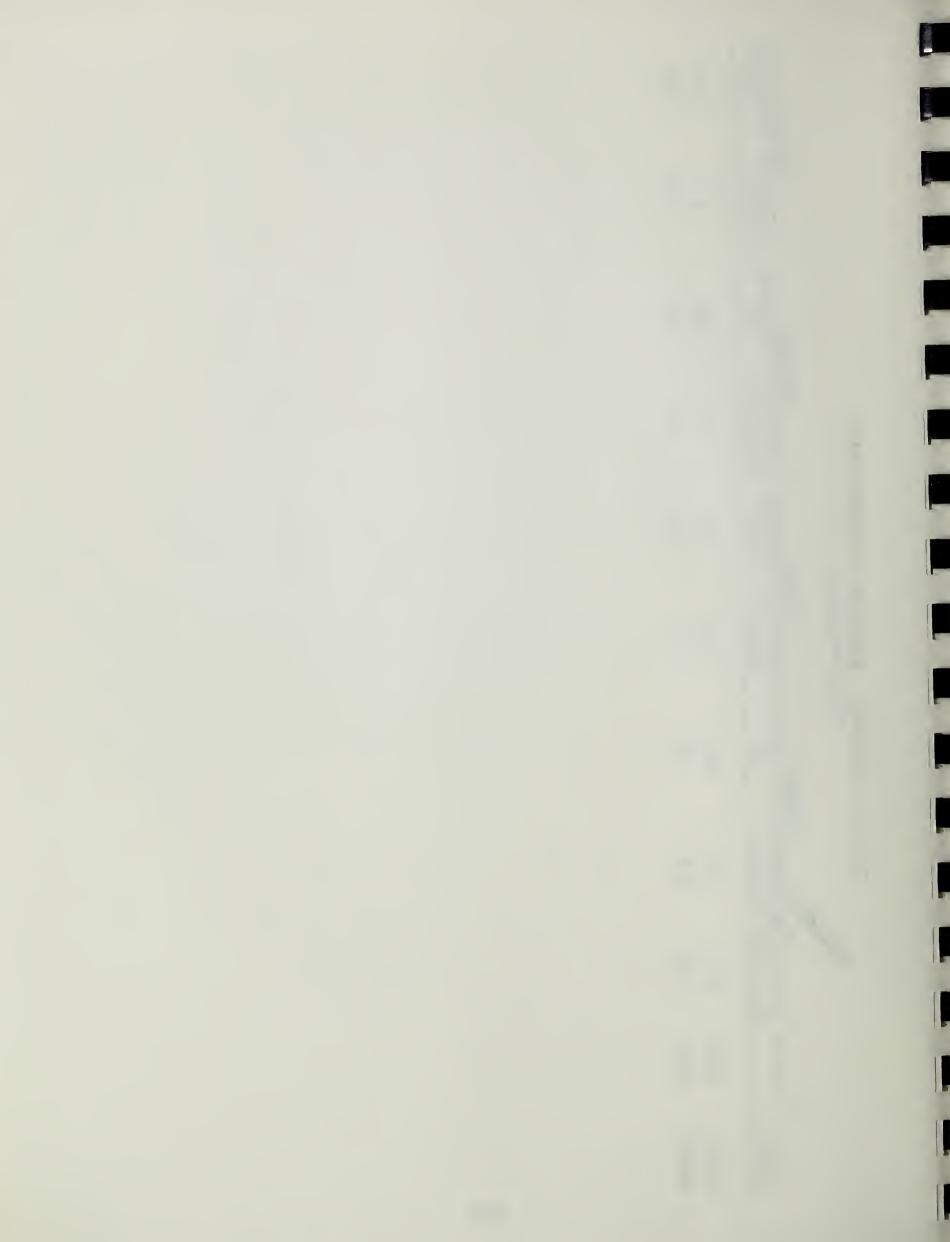


TABLE 11. DISCHARGE - ELEVATION - FREQUENCY DATA

Daughtrey Const.

Year	Discharge CFS	7288	7280	7261	7259	7242	6841	6820	6815	6741	6733	6611	6603	6299	6591	6229	6737	6526	6518	6514	6203	6457	6423	6400	6379	5984	5933
500-Year	Elevation: MSL-Ft	1.3	1.9	3.0	5.4	8.1	10.9	14.0	14.0	16.0	16.5	17.7	19.0	19.2	20.5	21.3	22.3	22.8	23.9	24.1	24.1	25.5	26.1	26.7	27.4	28.3	28.7
ear	Ulscharge CFS	5039	5034	5021	5019	5007	4723	4712	4705	4652	4647	4560	4554	4550	4546	4524	4508	4500	4494	4492	4488	4451	4427	4411	4396	4117	4081
100-Year	MSL-Ft. :	1.0	1.4	2.1	4.1	7.5	10.1	12.6	12.6	15.2	15.6	16.7	18.0	18.1	19.5	20.4	21.3	21.7	23.0	23.3	23.3	25.2	25.6	26.3	26.8	27.5	27.9
	Ulscharge : CFS	4148	4143	4132	4131	4121	3886	3878	3871	3827	3822	3751	3746	3744	3739	3720	3707	3701	3698	3694	3691	3660	3641	3627	3615	3384	3354
50-Ye	MSL-Ft. :	0.8	1.1	1.7	3.3	7.1	9.6	12.1	12.1	14.6	15.0	16.2	17.5	17.7	19.0	20.1	20.9	21.3	22.6	23.1	23.1	24.9	25.3	26.0	26.5	27.1	27.5
10-Year	Ulscharge CFS	2389	2386	2379	2379	2373	2234	2229	, 2225	2199	2196	2154	2151	2150	2147	2136	2129	2125	2122	2121	2119	2101	2089	2082	2074	1938	1921
10-1	(2) MSL-Ft. :	0.3	0.4	9.0	1.5	3.0	7.5	10.0	10.6	12.7	13.2	14.4	15.9	16.4	18.0	19.0	19.8	20.1	21.4	22.1	22.1	22.6	24.2	25.4	25.7	26.2	26.5
	Area (Mi2)	38.67	38.61	38.46	38.44	38.31	35.18	35.01	34.98	34.41	34.35	33.42	33.36	33.30	33.27	33.03	32.86	32.78	32.72	32.69	32.65	32.26	32.01	31.84	31.68	28.79	28.42
**************************************	- 1	0	069	1390	2140	3490	7090	7170	8540	11170	11940	12890	14390	14490	16270	16850	19160	20430	21710	21890	22090	23070	26710	27350	28335	29685	32285
Š	Section	DAU004	DAU037	DAU038	DAU040	DAU047	DAU050	DAU060	DAU065	DAU003	DAU076	DAU080	DAU085	DAU095	DAU100	DAU110	DAU002	DAU116	DAU120	DAU121	DAU122	DAU125	DAU140	DAU155	DAU166	DAU170	DAU001

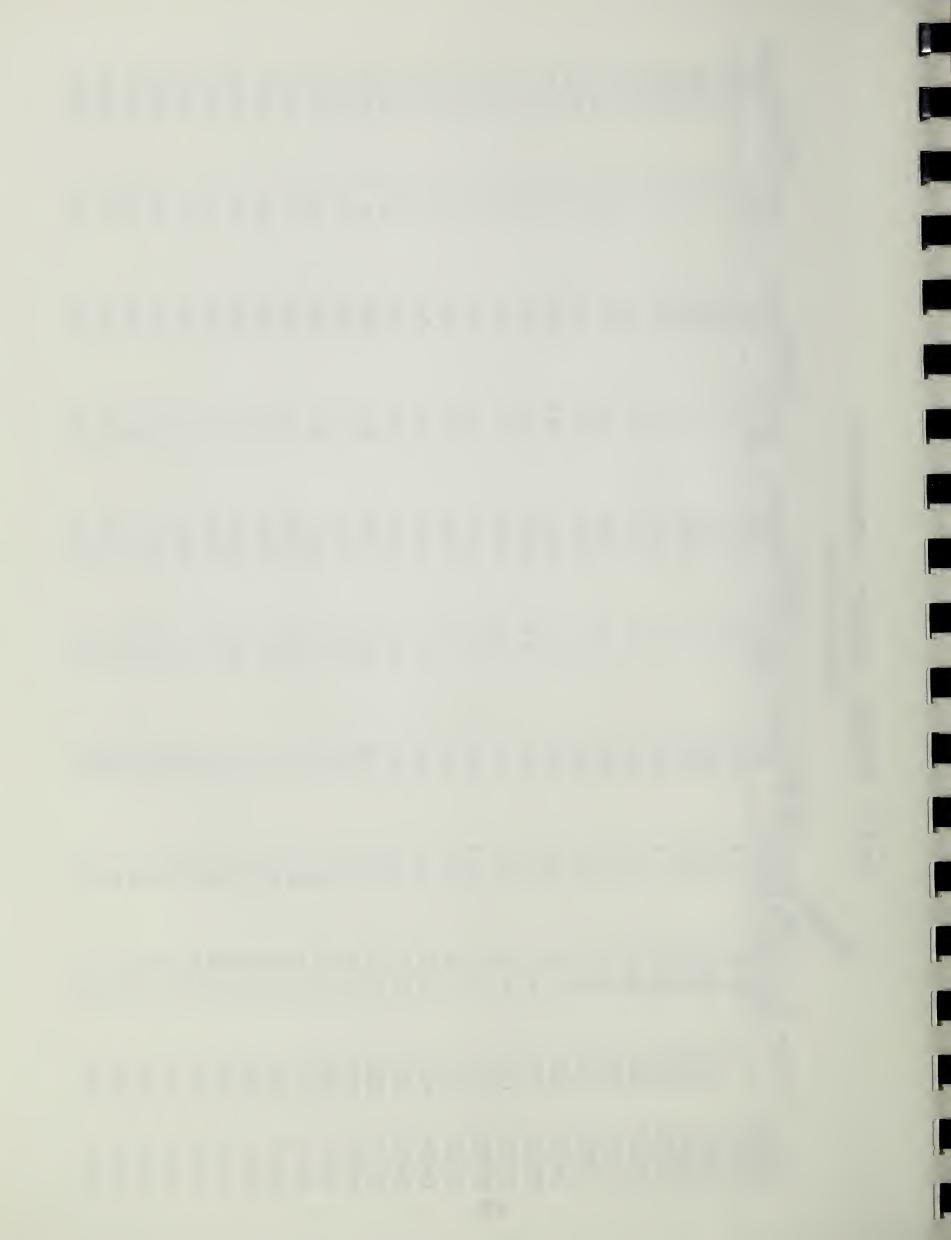


TABLE 12. DISCHARGE - ELEVATION - FREQUENCY DATA

Daughtrev Crook Tail (1)

	ge	1			
/ear	: Discharge : CFS	093	000	546	537
; 500-Year	Elevation: MSL-Ft	٦٤ ۶	0.0	16.7	17.1
Year	Discharge CFS	362	306	353	347
100-Year	Elevation: Discharge MSL-Ft.: CFS	15.7) l	2.8	16.5
	on: Discharge CFS	262	1 u	585	280
50-Year	Elevation: [MSL-Ft. :	15.2	ן כ ט ני	15.3	15.7
ar :	Discharge CFS	157		153	150
10-Year	Elevation: D MSL-Ft. :	13.4	בי כר	13.3	13.6
2	Drainage (Area (Mi ²)	0.83	00	0.00	0.78
•	Station iDrainage Station iDrainage	1230	0000	7700	2400
•	Cross :	DAU011	C LOUND	710000	DAU014

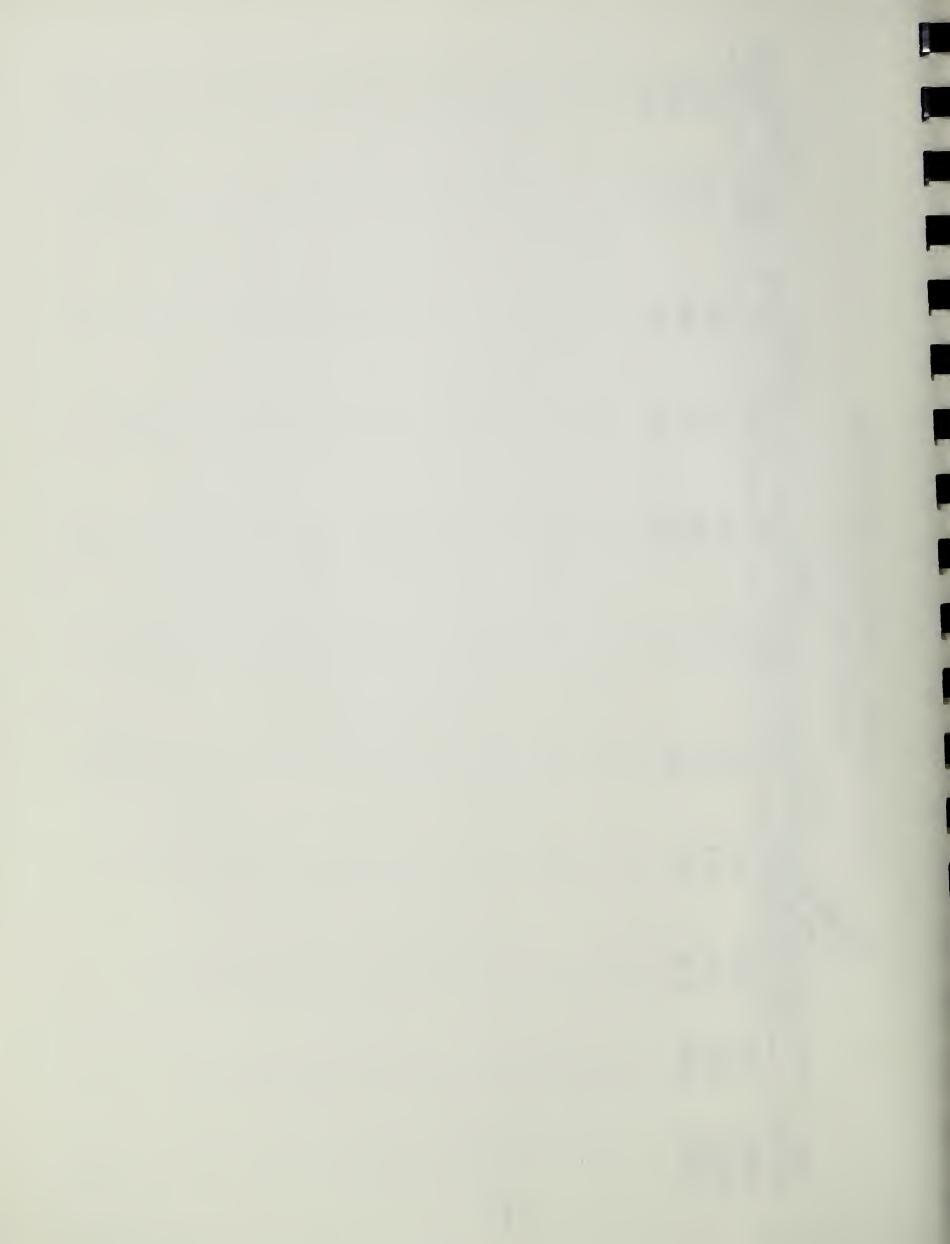


TABLE 13. DISCHARGE - ELEVATION - FREQUENCY DATA

100	·	ı
· · · · · ·	/ ('raak rih /	
	/ ロットロント	~ リーンニ・アラガロ

7 E G	Discharge CFS	355	251	
: 500-Year	Elevation: MSL-Ft	14.3	14.4	
/ear	Discharge CFS	227	159	
100-Year	<pre>Elevation: MSL-Ft. :</pre>	13.6	13.8	
ar	Discharge CFS	183	127	
50-Year	Elevation: MSL-Ft. :	13.0	13.2	
ear	Discharge CFS	97	67	
10-Year	# Elevation: MSL-Ft. :	11.0	11.2	
	Drainage	0.42	0.25	
	Statión Drainage	570	1770	
	Cross	DAU016	DAU017	

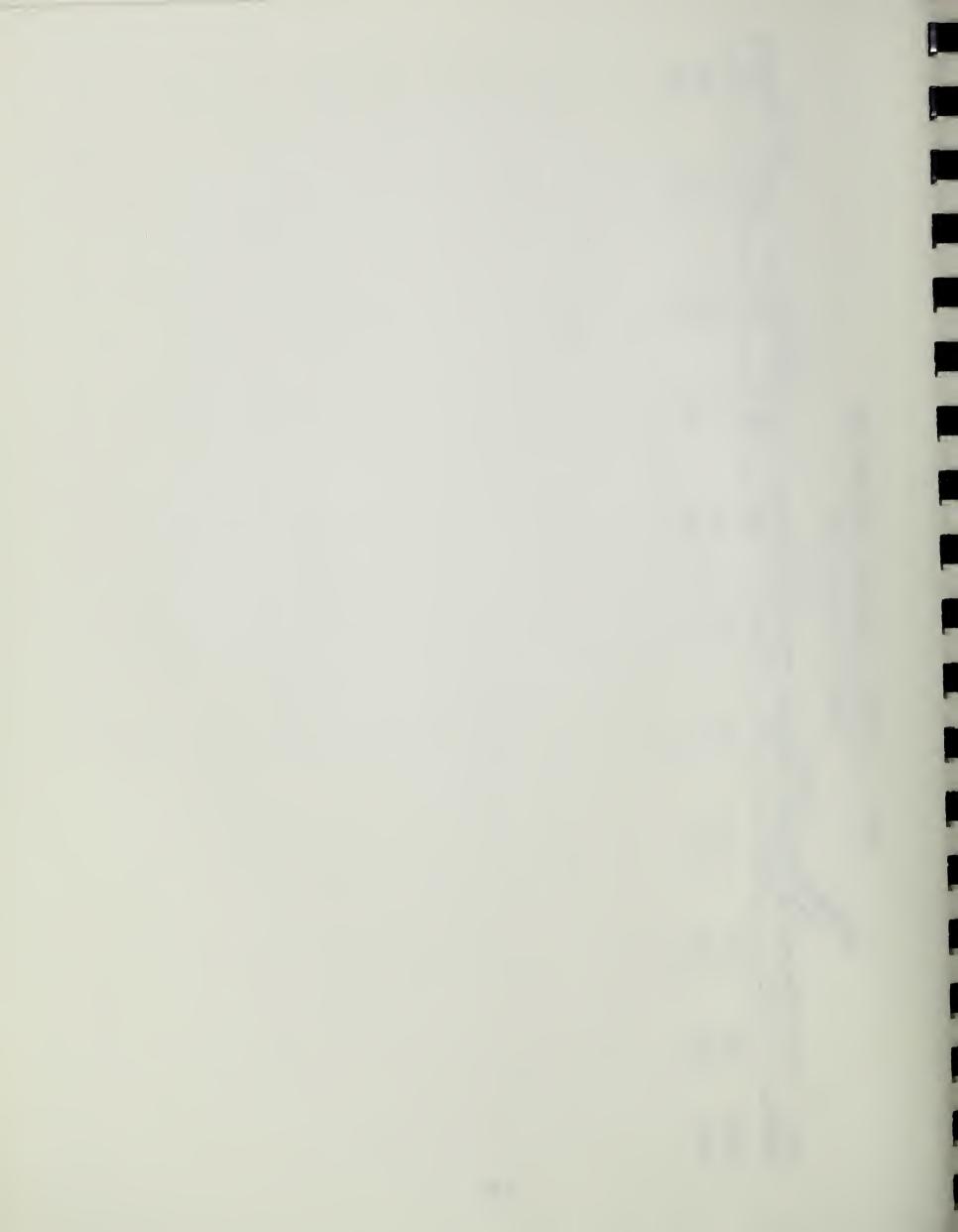


TABLE 14. DISCHARGE - ELEVATION - FREQUENCY DATA Daughtrey Creek Trib (Daughtrey East)

ar	ischarge CFS	1359	1315	1261	1219	1144	1041	1031	782	
500-Year	Elevation: Discharge MSL-Ft : CFS	8.3	9.1	11.4	15.0	16.3	17.2	18.2	22.3	
ear	Discharge CFS	006	870	834	805	755	685	829	511	
100-Year	Elevation: Discharge MSL-Ft. : CFS	7.7	8.4	10.9	14.8	16.0	17.0	18.0	21.9	
 L	Jischarge : CFS :	732	707	677	654	612	555	250	413	
50-Year	Elevation: Discharge MSL-Ft. : CFS	7.3	8.0	10.7	14.7	15.8	16.9	17.9	21.5	
ear		402	389	372	359	335	303	300	224	
2 10-Year	* Elevation: Discharge MSL-Ft. : CFS	3.7	6.4	9.1	14.1	15.4	16.7	17.5	20.9	
3070	Station Drainage Area (Mi ²)	3.13	2.98	2.80	5.66	2.42	2.10	2.07	1.37	
	Station	009	1775	3125	6325	8200	9700	10795	15745	
	Cross Section	DAU019	DAU023	DAU027	DAU031	DAU032	DAU033	DAU034	DAU035	

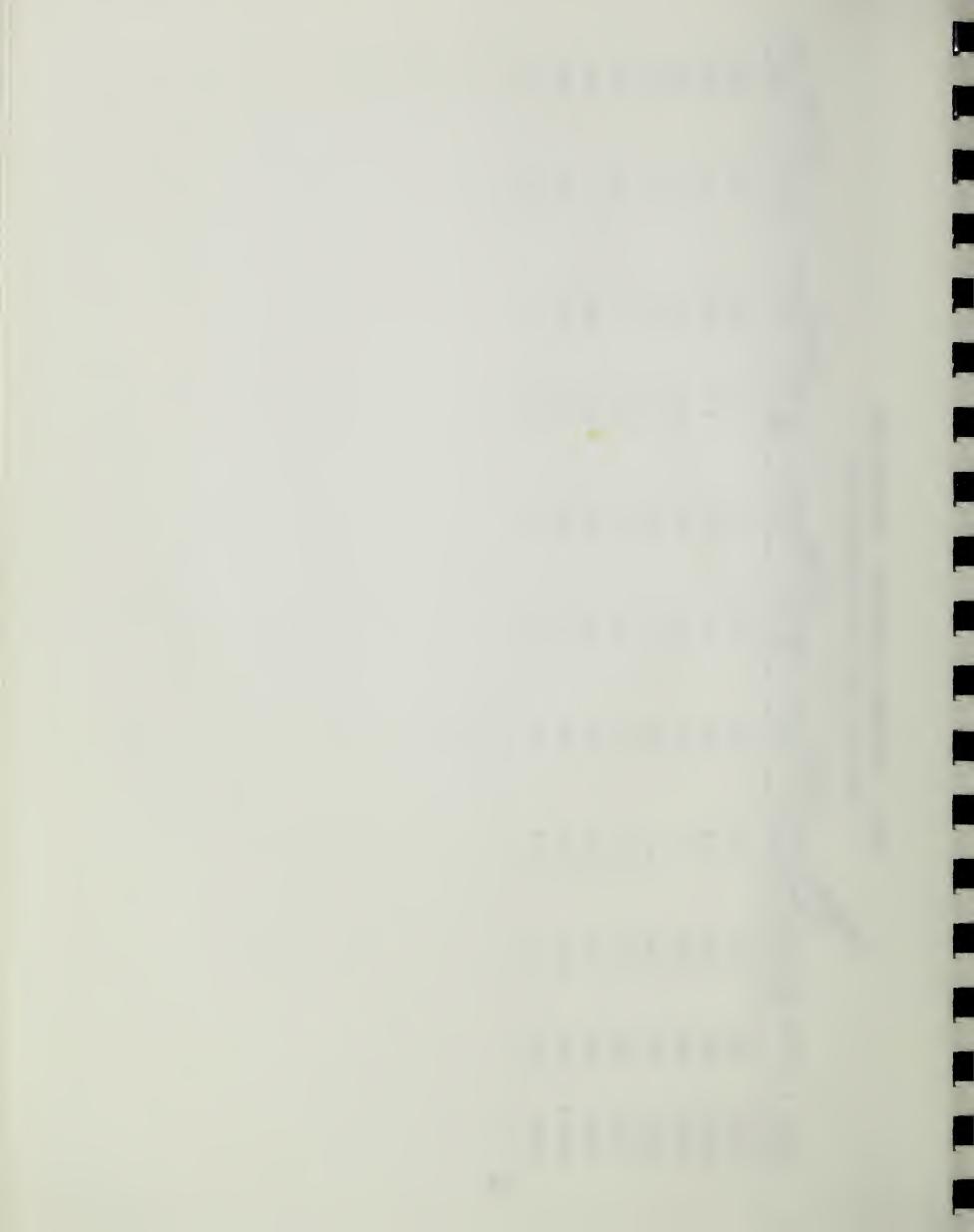


TABLE 15. DISCHARGE - ELEVATION - FREQUENCY DATA Chapel Branch

	4 + C + C		2	— [50-Ye	ar Jacobs woo	>-	ear	500-Y	ea r
Cross Section	station	Urainag Area (M	i2) MSL-Ft.	U1scharge CFS	MSL-Ft.:	Ulscharge CFS	MSL-Ft.:	Ulscharge :	Llevation: L MSL-Ft :	Ulscharge CFS
CHA037	1800	5.09	2.5	926	3.0	1396	3.2	1546	3.6	1934
CHA038	3000	2.05	4.3	926	4.8	1394	5.1	1544	5.6	1928
CHA040	5450	1.95	7.3	957	7.7	1389	ω _. ω _.	1537	8.9	1914
CHA046	5530	1.95	8.5	957	8.8	1389	6.6	1537	10.0	1914
CHA050	6230	1.89	8.6	957	8.9	1385	10.0	1533	10.0	1905
CHA060	6330	1.89	10.1	957	10.8	1385	11.0	1533	11.6	1905
CHA065	7680	1.65	10.1	926	11.0	1371	11.2	1515	11.9	1867
CHA075	7760	1.65	12.5	926	13.2	1371	13.8	1515	14.2	1867
CHA002	9260	1.52	14.3	855	14.6	1223	15.2	1350	15.8	1662
CHA081	9700	1.50	15.1	843	15.5	1201	16.2	1325	16.4	1631
CHA085	10870	1.30	15.5	693	16.1	984	16.8	1083	17.0	1333
CHA086	11320	1.28	16.5	678	16.9	963	17.1	1059	17.2	1304
CHA087	11600	1.23	16.9	642	17.1	116	17.2	1002	17.3	1232
CHA088	11910	1.18	17.0	209	17.2	860	17.3	945	14.7	1162
CHA089	12250	1.18	17.1	209	17.3	860	17.5	945	17.6	1162
CHA090	12860	1.11	17.2	558	17.5	790	17.6	867	17.7	1066
CHA100	12940	1.11	18.4	558	18.4	790	18.7	867	19.4	1066
CHA001	15040	. 68	19.2	467	19.5	. 229	20.0	753	20.1	943
CHA110	15620	. 65	19.4	460	19.8	899	20.2	743	20.3	932
CHA116	15660	.65	19.5	460	20.1	899	20.3	743	20.5	932
CHA117	16360	.62	19.6	452	20.3	658	20.5	733	20.7	921
CHA120	17690	.58	19.7	441	20.5	644	20.7	719	20.8	906
CHA130	17750	.58	19.9	441	20.7	644	20.8	719	20.9	906

TABLE 16. DISCHARGE - ELEVATION - FREQUENCY DATA

Bayshore Creek

ar ischarge CFS	1526	1525	1488	1488	1446	1431	1431	1423	1423	1412	1412	1403	1403	1403	1398	1398	1380	1373	1197	1197	1056	1056	882	882	821
Elevation: Dis	6.	4.1	5.4	7.0	8.5	11.3	11.5	11.6	12.7	13.7	13.8	14.2	15.2	16.0	16.9	. 18.0	19.0	19.4	19.8	20.1	20.5	20.9	22.0	22.0	23.0
rear Discharge CFS	1252	1251	1234	1234	1214	1206	1206	1202	1202	1198	1198	1164	1164	1164	1136	1136	1112	1078	940	940	831	831	695	969	647
100-Yea Elevation: Di MSL-Ft. :	2.6	3.7	. 5.2	6.9	8.3	10.9	11	11.4	12.6	13.6	13.7	14.1	15.1	15.9	16.8	17.9	18.9	19.3	19.7	20.0	20.4	20.8	21.3	21.9	22.9
scharge CFS	1126	1125	1112	1112	1098	1092	1092	1089	1089	1066	1066	1042	1042	1042	1025	1025	266	. 996	842	842	743	743	621	621	578
50-Year Elevation: Di MSL-Ft. :	2.4	3.4	5.0	6.7	8.1	10.4	10.9	11.2	12.4	13.4	13.6	14.0	15.0	15.8	16.7	17.8	18.8	19.2	19.6	19.9	20.3	20.7	21.2	21.8	22.7
Vear Discharge CFS	742	742	738	738	734	733	733	732	732	723	723	719	719	719	707	707	694	1/9	581	581	510	510	421	421	391
10- evation: SL-Ft.:	2.0	2.6	4.4	6.3	7.5	9.5	9.6	10.9	12.1	13.1	13.3	13.7	14.7	15.6	16.5	17.6	18.6	19.0	19.4	19.7	20.2	20.5	21.0	21.6	22.4
Drainage Ele	2.59	2.52	2.45	2.45	2.34	2.30	2.30	2.28	2.28	2.11	1.96	1.93	1.86	1.86	1.84	1.84	1.67	1.62	1.42	1.42	1.26	1.26	1.06	1.06	66.
Station	0	1560	2940	3040	4330	5110	5160	0909	6110	7340	7970	8610	9110	9190	10120	10220	13100	14000	15350	15410	17210	17260	18790	18850	19540
Cross	BAY004	BAY034	BAY035	BAY045	BAY049	BAY050	BAY060	BAY065	BAY075	B BAY003	5 BAY081	BAY082	BAY083	BAY084	BAY085	BAY095	BAY002	BAY101	BAY105	BAY115	BAY120	BAY130	BAY135	BAY145	BAY001

75

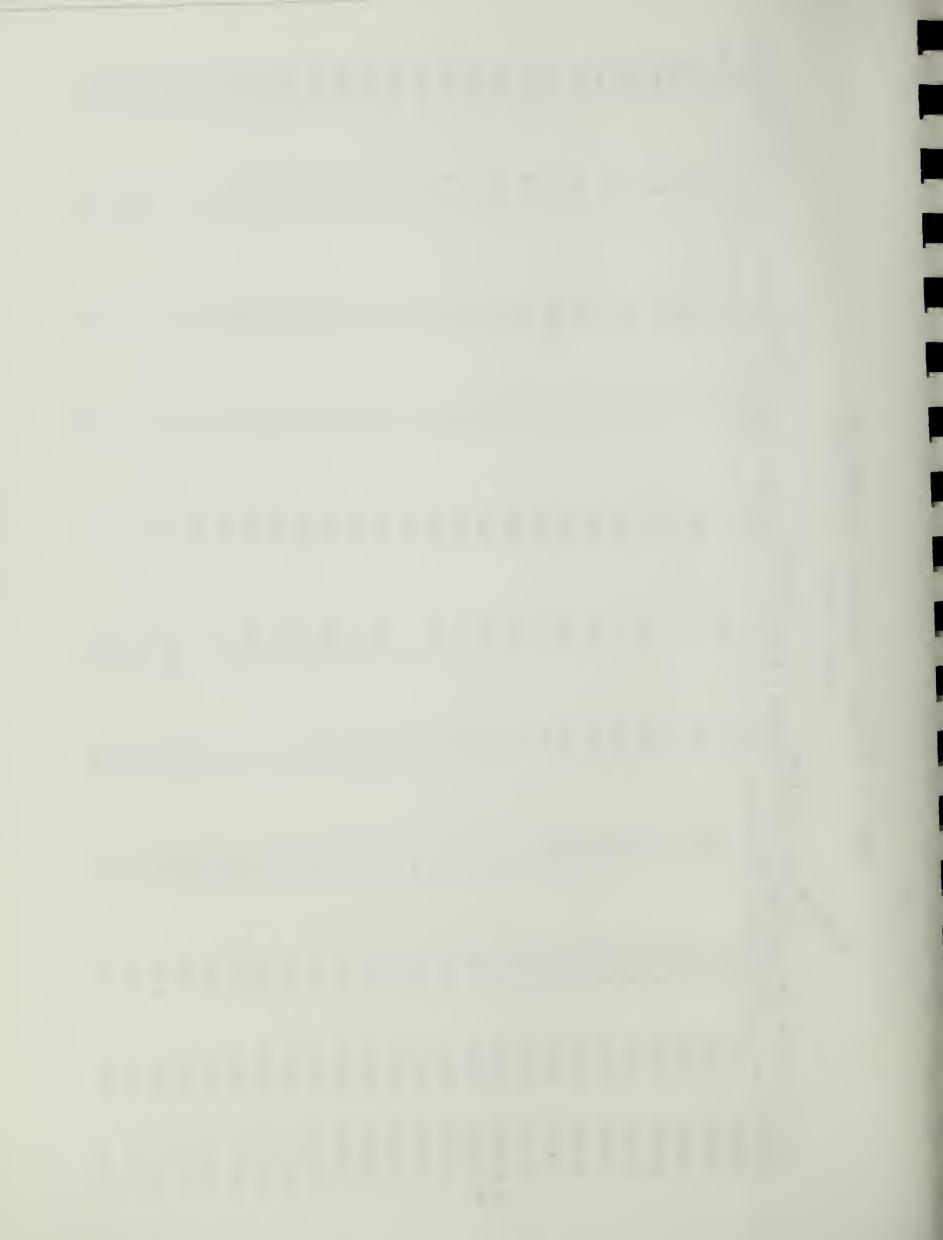


TABLE 17. DISCHARGE - ELEVATION - FREQUENCY DATA Bayshore Trib

CF (CF) (CF) (CF) (CF) (CF) (CF) (CF) (C	130
500-Year tion: Dischal Ft : CFS 3 195 7 134 8 133 9 131	-
500-Year Elevation: Discharge MSL-Ft : CFS 15.3 195 16.7 134 17.8 133 20.9 131	20.5
ischarge CFS 155 106 104	103
100-Year ion: Discrit. : Cl t. : Cl 5 10 7 10	=
100-Year Elevation: Discharge MSL-Ft.: CFS 15.2 155 16.6 106 17.7 106 20.5 104	20.3
0 00 00 00 00 0	
10 -	93
50-Year tion: Dis Ft. : 5 6	_
Elevati MSL-Ft 15.1 16.5 17.6 19.5	20.
rge	
MSL-Ft.: Discharge MSL-Ft.: CFS 14.8 87 16.0 66 17.2 66	64
10-Year tion: Disc Ft. : (C 8 8 8 0 0 6 2 6	വ
10- Elevation: MSL-Ft.: 14.8 16.0 17.2 19.2	19.5
0/ 0/ -	
Orainage Area (Mi ²) 0.08 0.08 0.03	0.03
Station Drainage (Mi ²) 105 0.08 175 0.08 915 0.03	1205
	65
Cross Section BAY151 BAY153 BAY155	BAY165

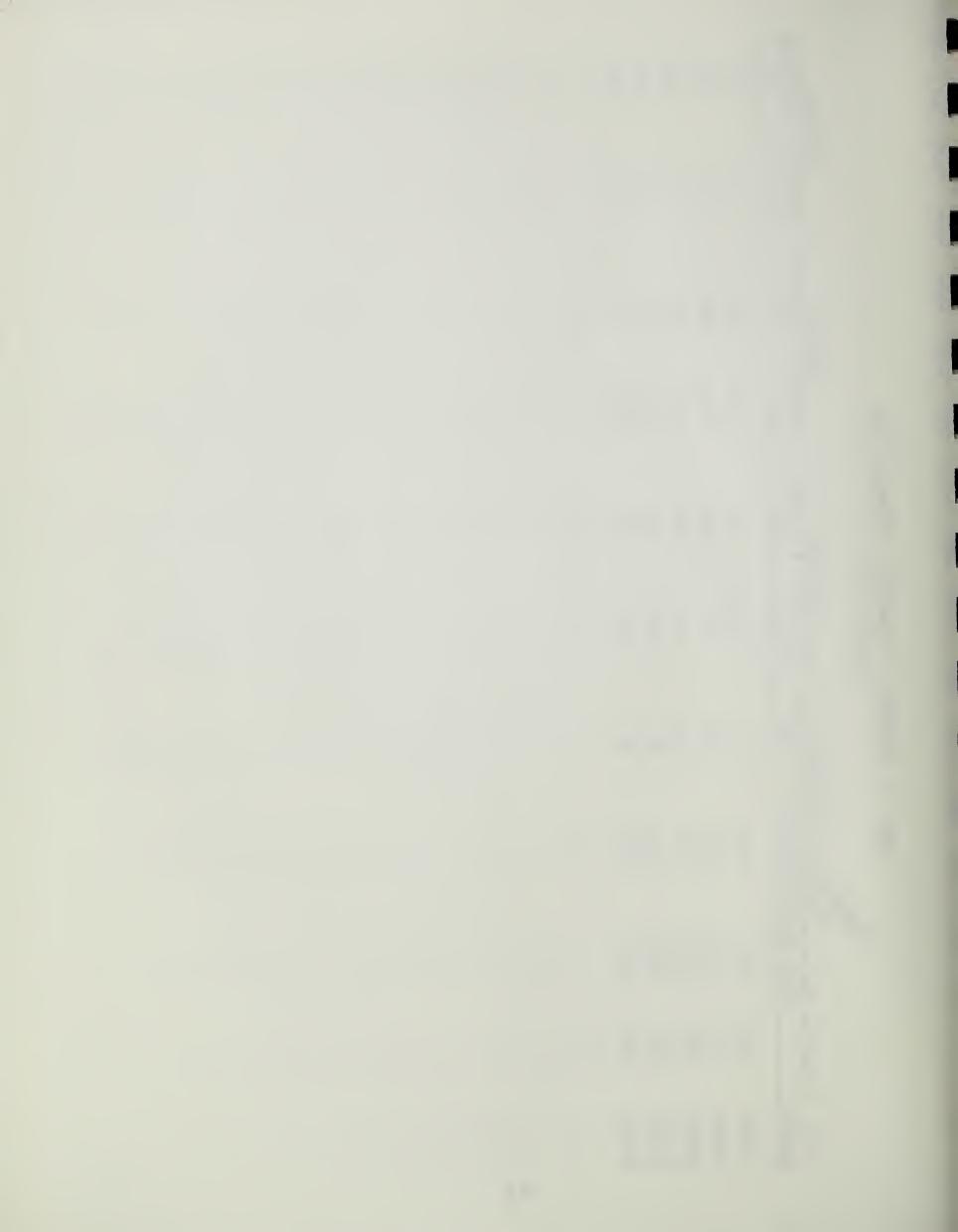


TABLE 18. DISCHARGE - ELEVATION - FREQUENCY DATA

ek
Cre
ے
pas
Po

(ear Discharge CFS	3643	3324	3206	2924	2924	2924	2923	2715	2160	1623	1586	1586	1479	1479	1479	1479	1478	1478	1478	1478	1478	1478	1478	1464	1464	1454	
Elevation: Dis	1.7	3.6	4.3	4.3	6.1	7.9	8.7	9.6	14.8	20.9	21.4	21.5	21.8	22.1	22.3	22.9	23.2	23.6	24.1	24.7	24.8	25.0	25.3	25.9	26.3	27.7	27.8
00-Year on: Discharge .: CFS	2792	2568	2485	2285	2285	2285	2285	2129	1713	1294	1265	1265	1181	1181	1181	1181	1181	1181	1181	1181	1181	1181	1180	1164	1164	1152	1147
Elevation MSL-Ft.	1.6	3.4	3.8	3.9	5.9	7.2	7.9	0.6	14.5	20.3	20.8	20.9	21.4	21.7	21.9	22.2	22.7	23.1	23.5	24.0	24.6	24.7	24.9	25.6	25.7	26.4	26.6
50-Year ion: Discharge t. : CFS	2450	2264	2195	2028	2028	2027	2027	1893	1532	1157	1132	1132	1057	1057	1056	1055	1055	1055	1054	1054	1054	1053	1053	1038	1038	1027	1019
Elevation MSL-Ft.	1.6	3.1	3.6	3.7	5.7	6.9	7.5	8.7	14.4	20.1	20.6	20.7	21.2	21.5	21.7	21.9	22.1	22.6	22.9	23.4	23.9	24.5	24.8	25.5	25.6	26.3	26.5
10-Year on: Discharge CFS	1585	1495	1461	1378	1378	1378	1378	1291	1056	787	692	692	755	755	755	754	754	753	752	739	729	720	720	902	902	269	969
e Elevation:	1.4	2.3	3.0	3.2	4.8	5.4	6.7	8.2	13.7	19.8	20.3	20.4	20.6	20.9	21.3	21.5	21.7	22.2	22.3	22.8	23.3	23.9	24.2	25.1	25.6	26.1	26.3
Drainage A.	8.55	8.29	8.19	7.94	7.94	7.94	7.94	7.63	6.75	6.23	6.19	6.19	6.07	5.99	5.99	5.59	5.59	5.37	5.37	5.22	5.19	5.01	5.01	4.59	4.59	4.30	3.88
Station	0	1450	3100	2600	0009	0919	6290	7890	11340	14140	15240	15320	16870	17520	17600	18080	18170	18740	18820	20140	20700	22980	23080	25630	26430	27980	28980
Cross Section	P0P066	P0P067	P0P068	P0P069	P0P070	P0P080	P0P090	P0P095	P0P100	4 P0P105	P0P110	P0P120	P0P125	P0P130	P0P140	P0P145	P0P155	P0P160	P0P170	P0P175	P0P180	P0P195	P0P005	P0P010	P0P015	P0P016	P0P020

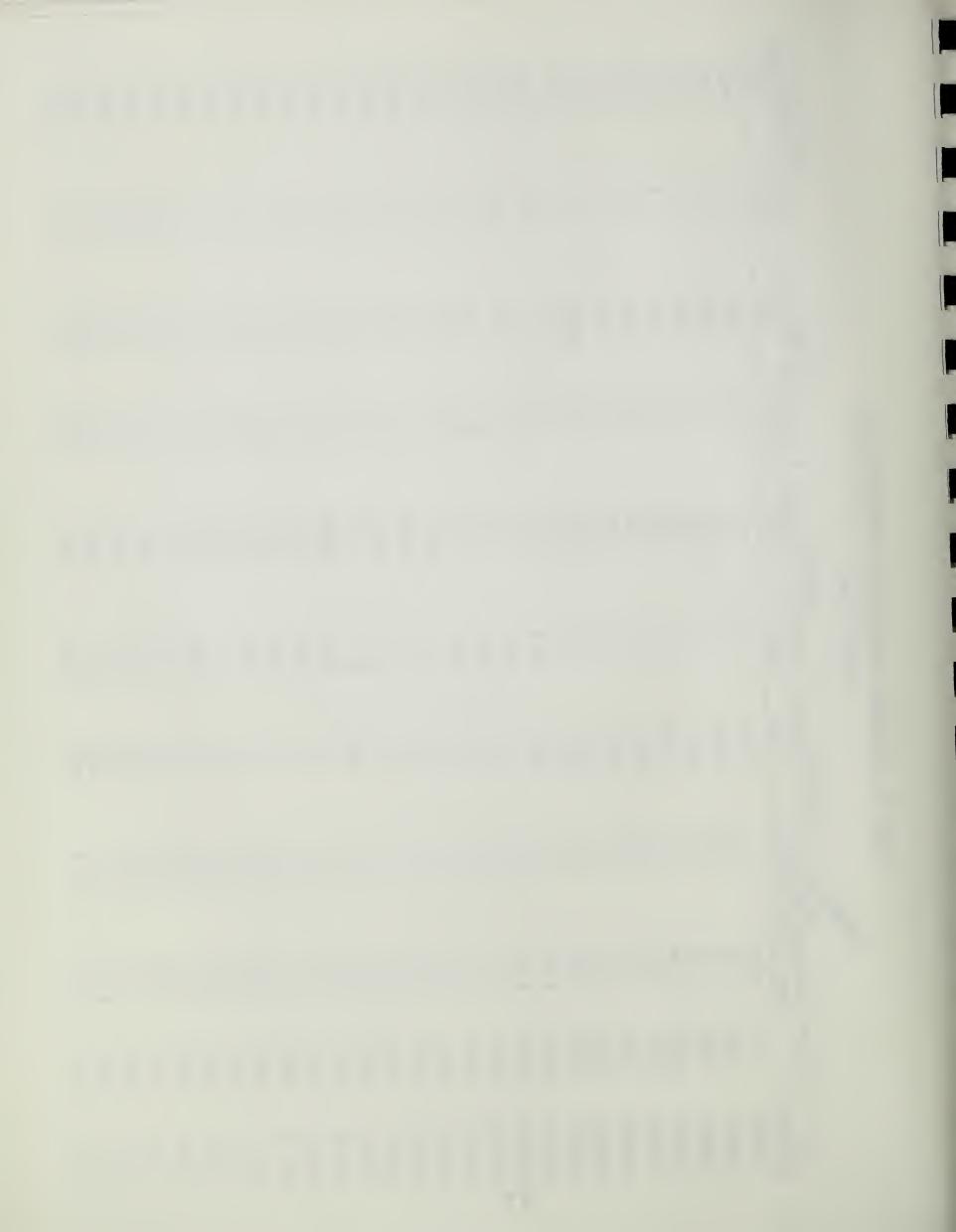


TABLE 19. DISCHARGE - ELEVATION - FREQUENCY DATA

Stroud Creek

Elevation: Discharge 3106 3072 2685 2685 2599 2526 2523 2724 2517 2517 500-Year MSL-Ft 4.9 5.7 8.6 10.9 16.2 8.7 18.4 Elevation: Discharge 2126 2125 2000 2423 2057 1995 1989 2157 MSL-Ft. 7.9 8.5 10.5 15.8 18.0 5.1 Elevation: Discharge 2163 1908 1843 1788 1778 1907 1937 1784 3.6 7.6 4.8 8.3 10.3 15.6 17.8 20.7 Drainage Elevation: Discharge Area (Mi²) A MSL-Ft. : CFS 1265 1219 1215 1209 1347 1487 1471 1321 1321 2.8 4.0 7.0 7.5 9.5 15.0 17.2 7.85 7.79 7.19 7.67 7.67 7.41 7.04 Station 1630 3580 4940 5090 6440 9240 1160 2590 3160 Section STD140 STD003 09LQLS **78** STD123 STD004 STD130 STD150 STD151 STD002 Cross STD124 S

2517	2516	2516	2509	2507	2478	2471	2462	2453	2410	1936	1930	1913
22.5	22.7	22.9	23.2	23.6	23.9	24.2	24.3	24.5	24.5	24.6	25.4	26.6
1989	1988	1988	1980	1978	1954	1949	1941	1934	1901	1527	1522	1509
21.9	22.1	22.4	22.7	22.9	23.1	23.4	23.5	23.6	23.6	23.7	24.3	25.6
1777	1777	1777	1769	1767	1745	1740	1733	1727	1697	1363	1359	1347
21.7	21.9	22.1	22.5	22.7	23.0	23.2	23.2	23.3	23.4	23.5	24.0	25.3
1209	1209	1209	1202	1199	1186	1183	1179	1174	1154	927	924	916
20.9	21.1	21.4	21.8	22.0	22.2	22.4	22.4	22.5	22.8	23.1	23.6	24.3
08.9	6.79	6.79	6.52	6.44	6.27	6.27	2.7	6.27	6.27	5.92	5.92	2.77
13220	13640	13690	14260	15580	17260	17660	17685	17710	18310	20210	20270	21590
STD170	STD175	STD185	STD190	STD001	STD009	STD010	STD015	STD020	STD021	STD025	STD035	STD036

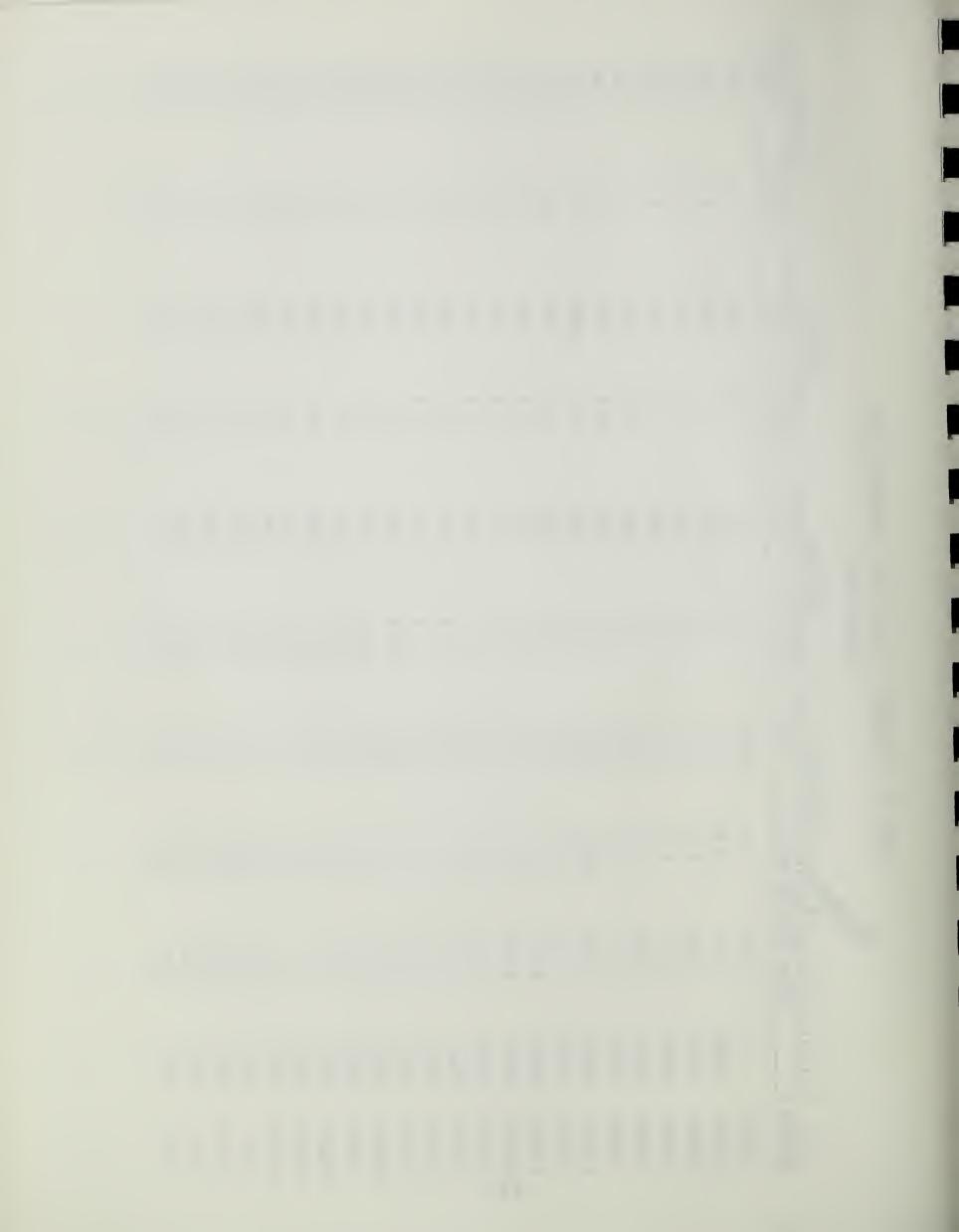


TABLE 20. DISCHARGE - ELEVATION - FREQUENCY DATA

Thompson Cutoff

504)	Station	Anainada	Ack 10-Year	ear Dischange	Flave	ear Discharge	100-Year		500-Year	2
Section	3	:Area (Mi ²)	1 ²) MSL-Ft. CFS		1	CFS		1	MSL-Ft :	CFS
T0M039	0	5.66	2.0	1593	2.23	2347	2.3	2625	2.6	3312
T0M003	300	2.63	2.2	1593	2.8	2347	3.0	2625	3.2	3312
T0M045	1650	2.56	8.8	1515	9.5	2245	9.4	2511	9.6	3170
T0M055	1750	2.56	10.0	1515	10.4	2245	10.6	2511	10.8	3170
T0M056	2620	2.01	10.9	196	12.0	1503	12.8	1687	13.0	2140
T0M002	3250	1.57	12.8	604	14.6	866	14.6	1124	14.8	1432
T0M065	3800	1.55	13.1	589	14.4	977	14.7	1101	15.0	1403
T0M075	3870	1.55	14.2	589	15.9	977	16.4	1101	16.8	1403
080W01 79	5520	1.49	16.4	547	16.9	915	17.0	1032	17.1	1316
T0M090	5580	1.49	17.1	547	17.9	915	18.1	1032	18.3	1316
T0M095	6540	1.44	17.5	513	18.2	865	18.3	975	18.6	1245
TOM105	0859	1.44	17.5	513	18.2	865	18.3	975	18.6	1246
T0M110	8470	1.33	18.8	442	19.8	758	20.1	928	20.2	1094
T0M120	8530	1.33	18.8	442	19.8	758	20.1	856	20.2	1094
T0M125	10090	1.25	19.6	393	20.1	684	20.1	773	20.2	686
T0M135	10170	1.25	19.6	393	20.1	684	20.1	773	20.2	686
TOM136	10420	1.04	19.7	278	20.1	504	20.1	571	20.2	733
T0M145	10490	1.04	19.7	278	20.1	504	20.1	571	20.2	733
TOMOOT	11530	0.91	19.8	216	20.1	404	20.1	459	20.2	290
T0M155	12740	08.0	20.3	205	20.5	373	20.5	423	20.5	543
T0M161	12850	08.0	21.1	205	21.6	373	21.9	423	22.5	543
T0M165	16120	0.25	23.8	124	24.5	182	24.9	203	25.0	257

79

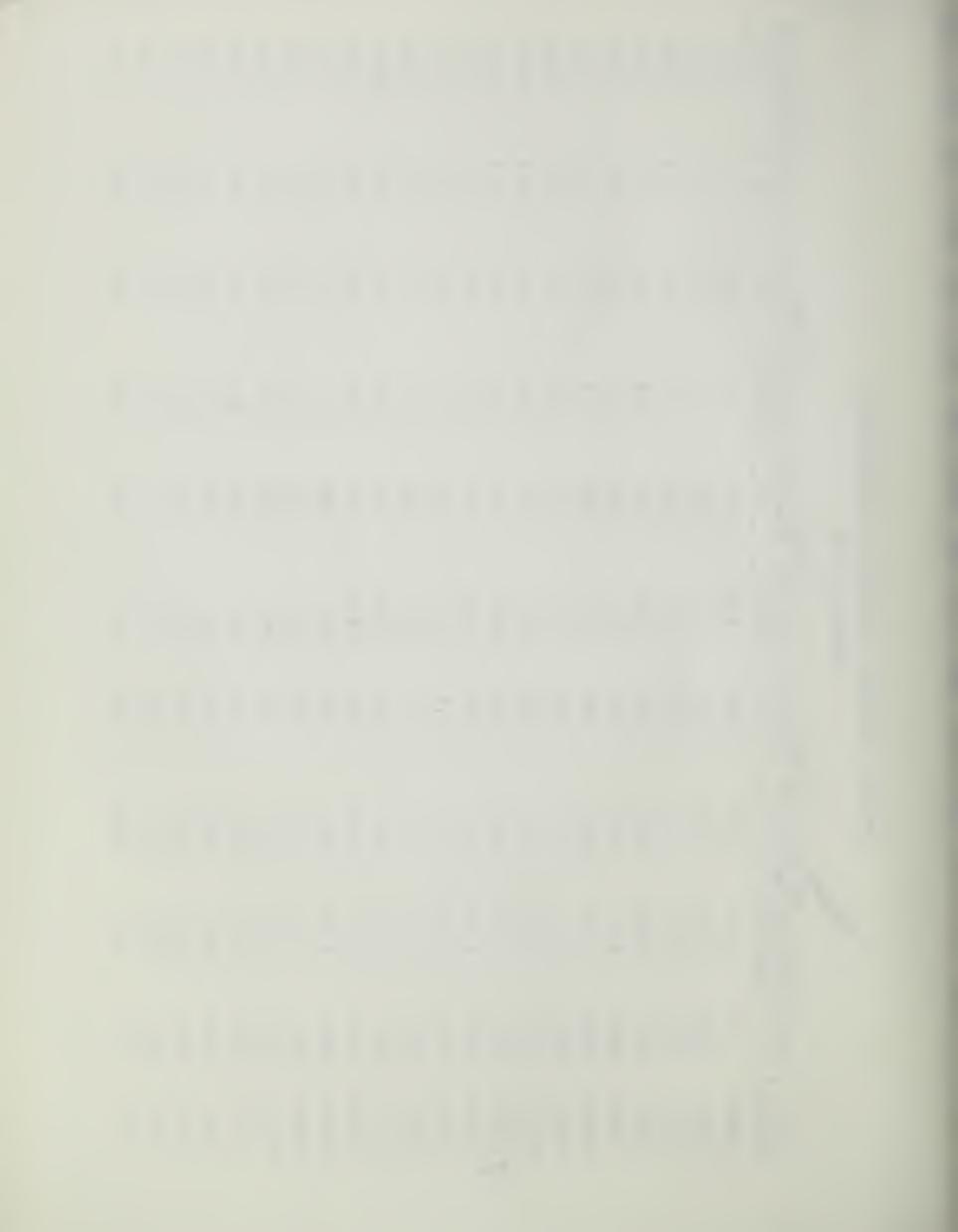


TABLE 21. DISCHARGE - ELEVATION - FREQUENCY DATA

Thompson Cutoff Trib

	•	9	10-Year	ear :	50-Year	ır .	100-Year	'ear	500-Year	2 0
Cross	Station	Drainage 2 Area (Mi	: Station Drainage of Elevation: Discharge : Area (Mi ²) MSL-Ft. : CFS		Elevation: [MSL-Ft. :	tion: Discharge Ft. : CFS	Elevation: Discharge MSL-Ft. : CFS	Discharge CFS	Elevation: Discharge MSL-Ft : CFS	Discharge CFS
T0M174	096	0.39	14.2	102	15.7	180	16.4	225	16.8	350
T0M173	1470	0.37	16.4	66	16.9	151	17.1	179	17.3	200
T0M172	2880	0.33	17.1	85	17.4	141	. 17.6	172	17.7	276
T0M171	3950	0.29	18.2	11	18.5	131	18.8	164	19.0	254
T0M170	5265	0.21	19.1	63	19.4	123	19.6	155	20.0	245
T0M169	6575	0.07	20.6	32	20.9	46	21.3	52	21.7	65

